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Our ref: JDV/jdv/19146

20 May 2022

By e-mail (rhuschan@hessequa.gov.za)

Attention: Mr. Werner Manho

Dear Sir

PROPOSED FLEUR DE VIE DEVELOPMENT, STILL BAY ON PORTION 1 OF THE FARM DUINEKROON NO. 591, DEVISION RIVERSDALE

Attached please find herewith the electrical services report for the abovementioned development, based on the discussions that this office has had with Mr Innocent Tererayi of Hessequa Municipality's Electrical Department, in Still Bay.

We would appreciate it if you could provide this office with feedback regarding the requests under Item No.'s 7.0 and 14.0 where the Capital Contributions be used to fund the external municipal connection, including the request that the Phase 2 connection be done when 50% of the residential house connections have been made.

Should you need to discuss the above to obtain more clarity in this regard please do not hesitate to contact me.

Yours faithfully

kOilligs

J.S. de Villiers Pr Tech Eng CLINKSCALES MAUGHAN-BROWN

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ELECTRICAL SERVICES REPORT

FOR THE

PROPOSED DEVELOPMENT ON REMAINDER OF THE FARM DUINEKROON NO. 591, DIVISION RIVERSDALE

FOR

TREVEE INVESTMENTS (PTY) LTD

REPORT NO: G/19146/E/R1

MAY 2022

CLINKSCALES MAUGHAN-BROWN CONSULTING MECHANICAL & ELECTRICAL ENGINEERS



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1.0 **INTRODUCTION**

This report has been prepared by Messrs. Clinkscales Maughan-Brown, who has been appointed by the Developer, Trevee Investments (Pty) Ltd, as the Electrical Consultants for this project. The purpose of this report is to provide the necessary information on the proposed electrical supply to and inside this Development and the connection to the main infrastructure in the area in order to obtain approval from the Supply Authority.

2.0 **<u>THE AREA</u>**

The property to be developed is Remainder of the Farm Duinekroon No. 591, Division Riversdale, in the location as indicated on the attached Drawing No. 19146/E/01. The development includes one hundred and forty six (146) General Residential Zone II (Group Housing) erven, one (1) General Residential Zone IV erf (approximately 35 units), and one (1) Business Zone III erf.

3.0 SUPPLY AUTHORITY

The Supply Authority for the area is Hessequa Municipality, and therefore their Electricity Department was consulted on all matters related to the electrical services.

4.0 BASIS OF REPORT

The report is based on the following:

- (i) Discussions with Mr. Innocent Tererayi at the Electricity Department at Hessequa Municipality.
- (ii) The layout drawing received from Planserv.
- (iii) Investigation of the existing and master MV (11kV) municipal primary network in the area.

5.0 **DRAWING**

Drawing No. 19146/E/01, which is a plan layout, also depicts the following:

- (i) The extent of the existing MV municipal primary network in the area.
- (ii) The two Connection Points to the existing Municipal network.
- (iii) The external and internal reticulation to and inside this development.
- (iv) The external reticulation that is requested to be funded with funds from the Capital Contribution Levy.
- (v) The external reticulation that is requested to be funded with funds from the Capital Contribution Levy by the other adjacent future Developers.

..../2

6.0 **CONNECTION POINTS**

It is proposed that the connection point be the MV underground cable between SS-Main Intake and SS-3 in the approximate location as depicted on Drawing No. 19146/E/01. A ring main unit, i.e. RMU No. 1 will be cut into the underground cable as depicted on the aforementioned drawing.

The second Connection Point will be the 11kV busbars where two 11kV circuit breaker panels will be connected.

7.0 EXTERNAL CONNECTION

It is proposed that the external connection to this development be completed in the following phases as depicted on Drawing No. 19146/E/01:

Phase 1:

The supply and installation of Ring Main Unit No.'s 1 and 2, including the MV underground cable between the aforementioned ring main units indicated in pink on the aforementioned drawing.

Phase 2:

The supply and installation of the Circuit Breaker Panel No.1 in the Main Intake Substation, including the MV underground cable between the aforementioned circuit breaker panel and Ring Main Unit "RMU No. 2". This work is requested to be completed when 50% of the 146 General Residential Zone II (Group Housing) erven have been registered. It is further requested that these works, indicated in green on the aforementioned drawing, be funded from the Capital Contribution received from the Developer. Should the Capital Contribution not cover the installation the balance will be funded by the Developer.

Phase 3:

The supply and installation of the Circuit Breaker Panel No. 2 in the Main Intake Substation, including the MV underground cable indicated in blue between the aforementioned circuit breaker panel and Ring Main Unit "RMU No. 1", which will ensure a ring feed supply to this and the adjacent future developments. It is requested that this work be funded from the external connection costs and Capital Contributions received from the adjacent Developers.

8.0 TAKING-OVER OF INTERNAL INSTALLATIONS

The Municipality will take over the complete external and internal reticulation, i.e. circuit breaker panels in the Intake Substation. MV underground cables, ring main units. miniature substations, LV distribution kiosks, streetlights, LV underground cables and service connection installations to the erf boundary of each consumer. For this reason the complete electrical installation would have to comply with the technical requirements of the Municipality and their supply conditions.

9.0 **TECHNICAL PARTICULARS**

All MV and LV cabling will be underground. Ring Main Units and Miniature Substations will be plinth mounted. Distribution kiosks, located next to the roads, will be used to house the consumer circuit breakers. Each consumer circuit will be metered via a pre-payment meter inside each building. Provision will, however, be made for space allowance for a single / three phase conventional meter inside the aforementioned distribution kiosks should the Municipality decide to approve the supply and installation of conventional meters at a later stage. Streetlight fittings will be supplied and installed along the roads. Provision has been made for LV bulk supply points to the General Residential Zone IV erf (30 flats), and one (1) Business Zone III erf.

The complete electrical installation, incl. streetlights, would have to comply with the technical requirements of Hessequa Municipality and their supply conditions

The plans and specification will be submitted to the Municipality for their records and approval.

10.0 ENVIRONMENTAL MANAGEMENT PLAN

All work on site will comply in all respects with the environmental management requirements.

11.0 SWITCHING OF SUPPLIES AND APPROVALS

No switching of supplies or trenching adjacent to existing cables will be carried out without prior arrangement with the Town Electrical Engineer's Department. The Electrical Contractor will also liaise with the Town Engineer's Department and Telkom to ensure that no damage is caused to existing underground piped services during construction.

12.0 **DEMAND**

Based on the information presently available, the peak kVA demand of this development has been calculated as follows:

146 General Residential Zone II (Group Housing) erven @ 10,35kVA	
(45A single phase) x 0,3 diversity factor =	454 kVA
1 General Residential Zone IV erf (35 units), @ 10,35kVA (45A single	
phase) per flat x 0,3 diversity factor =	109 kVA
(1) Business Zone III erf =	200 kVA
Others, streetlighting, pumpstations etc. =	<u>50 kVA</u>
	813 kVA

This is a provisional calculation and will be finalised after all the network load particulars have been concluded.

13.0 **PROGRAMME**

It is expected that the installation of services at the development will commence immediately after all the necessary approvals have been received.

14.0 **CAPITAL COSTS**

The following assumptions are being made:

- (i) The Developer will be responsible for the supply, installation and commissioning of the external MV and internal LV installation and connection to the external network as indicated on the attached drawing. This work will be done under the direction of an Electrical Engineer and Electrical Contractor to be approved by the Municipality.
- (ii) The Capital Contribution will be calculated in terms of the standard municipal policy in this regard. It is requested that External MV cable No.1, including the Circuit Breaker No. 1 Panel inside the Main Intake Substation be funded from the aforementioned capital contribution.

It is requested that the costs of External MV Cable No. 2, including the Circuit Breaker No.2 Panel inside the Main Intake Substation be funded from the other nearby Developers in future.

15.0 CONCLUSION

We trust that this information is sufficient for the Municipality to draw-up the Services Agreement.

Yours faithfully

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J.S. de Villiers Pr. Tech. Eng. CLINKSCALES MAUGHAN-BROWN

ANNEXURE A:

Drawing No. 19146/E/01



FLEUR DE VIE LIFESTYLE ESTATE

SERVICES REPORT FOR CIVIL ENGINEERING SERVICES FOR THE DEVELOPMENT OF PORTION 1 OF THE FARM DUINEKROON NO. 591, STILL BAY WEST

HESRIV-515

Revision 0.0

JUNE 2022

PREPARED BY:



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CLIENT:

Trevee Investments (Pty) Ltd



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1. INTRODUCTION

Hessequa Consulting Engineers CC has been appointed by the Developers of Trevee Investments (Pty) Ltd for the planning of civil engineering services for the proposed development of Portion 1 of the Farm Duinekroon No. 591, Still Bay West.

The provision of civil engineering services will be in accordance with the guidelines and requirements of the *Guidelines for the Provision of Engineering Services and Amenities in Residential Township Development* as published by the CSIR and those of the Hessequa Municipality.

This report indicates, discusses and elaborates on the design criteria and specifications to be applied in the detail design of the internal and external infrastructure including roads, stormwater, water and sewer reticulation as well as requirements for the provision of communication and electrical sleeves.

2. LOCATION AND ACCESS

Portion 1 of the Farm Duinekroon No. 591 is located to the south eastern side of Buitekant Street (Gravel portion) in Still Bay West.



Access to the proposed development will be from an extension of the existing paved road. (Extension of Buitekant Street) The Developer will extend the Public Road parallel to the eastern boundary up to the south eastern boundary peg of the development. Hessequa Municipality plans to extend the Public Road, from the south eastern boundary, to intersect with a future extension of Thys Vissie



Street, and ultimately joins with Bosbokduin Avenue. The proposed development will not have direct access from the planned Western Arterial.

SMEC Consulting Engineers compiled a Traffic Impact Assessment (TIA – Reference No. C1880 Revision 1 dated 6 May 2022) for the proposed development. The recommendations underlined in the TIA must be read in conjunction with the Traffic Master Plan, for Still Bay, as prepared by Lyners Consulting Engineers. The following recommendations are made with regard to access to the development as well as the upgrading of other related road infrastructure:

- The roadside environment has been identified as a suburban area. It is recommended that the access spacing requirements of a Class 5 Local Street be applied to the Public Road extension.
- Access to the development not to be closer than 105m from the Buitekant Street intersection from the new Public Road.
- The intersection between Main Road and Buitekant Street to be upgraded to a roundabout with two circulating lanes. The north approach to comprise of a shared through-and-left lane plus a short shared through-and-right lane. The east approach to comprise of a single lane serving all movements. The south approach to comprise of a shared through-and-left lane plus a short shared through-and-right lane. The west approach to comprise of a single lane serving all movements.

It is proposed that the Developer and Hessequa Municipality negotiate the incremental impact and cost related to the incremental impact for the required roundabout upgrading in relation to the Road Master Planning.

3. ENGINEERING SERVICES

Civil engineering services will be designed in accordance with the design standards of the *Guidelines for the Provision of Engineering Services and Amenities in Residential Township Development* as published by the CSIR as well as the minimum requirements of Hessequa Municipality.

3.1. MASS EARTHWORKS

No mass earthworks are envisaged.



3.2. ROADS

The access road will consist of 2 x 4m wide lanes with a 3m wide island within the proposed 13m road reserve. Road finishing will consist of 80mm Interlocking segmented paving with stormwater pipework and inlet- and outlet structures. In areas with steep slopes (12% and more) cross beams will be constructed at 15m intervals.

The design criteria will be based on the design standards of the *Guidelines for the Provision of Engineering Services and Amenities in Residential Township Development* as shown in Table 1 below.

Table 1 Road Design Criteria				
Parameter	Local Street	Residential Access Loop	Residential Access way	
	(Class 5)	(Class 5b)	(Class 5d)	
Category	UB	UC	UC	
Traffic Class	E1	E0	E0	
Structural Design Traffic	0.05 - 3 x 10 ⁶	< 0.2 x 10 ⁶	< 0.2 x 10 ⁶	
Surface Treatment	80mm Interlocking Segmented Paving	80mm Interlocking Segmented Paving	80mm Interlocking Segmented Paving	
Sub-base from commercial sources	150mm G5 (95% MAASHTO) on 150mm Upper Selected (93% MAASHTO) on 150mm Roadbed prep in-situ Material (90% MAASHTO)	150mm G5 (95% MAASHTO) on 150mm Upper Selected (93% MAASHTO) on 150mm Roadbed prep in-situ Material (90% MAASHTO)	150mm G5 (95% MAASHTO) on 150mm Upper Selected (93% MAASHTO) on 150mm Roadbed prep in-situ Material (90% MAASHTO)	
Geotechnical Report	-	-	-	
Carriage Way Width	6m	5,5m	5m	
Design Speed	50 km/h	40 km/h	30 km/h	
Maximum Gradient	10% over 100m max	16% over 50m max	16% over 30m max	
Minimum Gradient	0.5%	0.5%	0.5%	
Cross Fall	2,5%	2,5%	2,5%	
Bell mouths	8m Radius	8m Radius	8m Radius	

A Geological Report will be completed prior to the commencement of the design stage. Pavement design will be done in accordance with the Urban Transport Guidelines, UTG2, Structural Design of



Segmental Pavements for Southern Africa, 1987. Suitable commercial sources for construction materials are available in Still Bay and surrounding towns.

3.3. STORMWATER

3.3.1. Stormwater Management Strategy

It is estimated that stormwater runoff, depending on erf coverage, will increase by approximately 25% post development. The site drains in a north eastern direction. Stormwater will cross the new Public Road and drain onto existing linked open park areas, between existing township development, which ultimately reaches and releases into the Goukou River.

The following measures are proposed to mitigate the impact of post development stormwater runoff on the existing infrastructure downstream from the proposed development:

- Installation of 2 kl water tanks on each residential erf will contribute to the attenuation of initial runoffs.
- Public Open Spaces will be utilised as recreation areas as well as stormwater detention areas where the concentration of stormwater runoff will be minimised through the application of landscaping techniques, i.e. by creating grass lined swales, undulations and depressions.

3.3.2. Stormwater Design

Stormwater infrastructure will be constructed in accordance with the standard requirements and specifications as agreed with the Hessequa Municipality.

Stormwater runoff from the erven will gravitate towards the internal road network. Surface runoff from roadways will be collected via kerb inlets into a piped stormwater system. Servitudes have been provided for stormwater infrastructure and drainage as required.

Attenuation areas will, as far as possible, be created on open spaces and at stormwater pipework outlets, to prevent any negative effect on lower lying properties.

Design criteria adopted for the development with regard to stormwater infrastructure is summarised as follows:

Runoff rates will be determined according to the Rational Method.

Flood recurrence interval	:	2 years
Pipe material	:	Concrete
Pipe class	:	75D / 100D

SERVICES REPORT FOR CIVIL ENGINEERING SERVICES FOR THE DEVELOPMENT OF PORTION 1 OF THE FARM DUINEKROON NO. 591, STILL BAY WEST



Pipe diameters	:	Minimum 375mm \emptyset up to diameter as required
Bedding	:	Class C
Inlets	:	Kerb and drop inlets as required
Manholes	:	Point of deflections on pipes

3.4. WATER

3.4.1. Water Demand

Total AADD	l11,75 kℓ/day or 1,29 ℓ/s
1 Business Zone III : 0,3915 ha @ 18 kł/d/ha	7,05 kł/day
38 General Residential Erven Zone IV (Flats) @ 450 t/unit/day	7 17,10 kł/day
146 General Residential Erven Zone II @ 600 t/unit/day	87,60 kł/day
The estimated Annual Average Daily Demand (AADD) for the	development is as follows:

3.4.2. Water Sources for Still Bay West

The current peak weekly demand for Still Bay is 1,7 to 1,8 times the AADD and is experienced during December. The current peak week flow for Still Bay West is 53,0 ℓ /s. The proposed full development requires a peak weekly demand of 1,29 x 1,75 = 2,258 ℓ /s. Hessequa Municipality, in June 2022 completed a water source audit to confirm the yields of existing bore holes and fountains. The development of water sources is an ongoing exercise which is driven by Hessequa Municipality and funded through Capital Contributions payable per erf.

Table 2 : Water Sources for Still Bay West		
SUPPLY SOURCE	DELIVERY (ℓ/s)	
Olienhoutfontein	5	
Grootsandfontein	10	
Attie Nel	9	
Hawefontein	7	
Palinggatfontein	7,5	
Gholfbaan Borehole	5	
Olive Grove Dam	11	
Total Still Bay West	54.5	



3.4.3. Water Storage

The proposed development falls within the Plattebosch district and require storage capacity of 224 kł which is 48 hours of the AADD. The existing storage capacity at Plattebosch is 6,84 Mł with a Full Water Level (FWL) of 108m. The proposed development has an elevation range between 49,5 and 63 m a.s.l.

In accordance with the GLS Report dated 19 April 2022 sufficient storage capacity exists, within the Plattebosch reservoir district, to accommodate the proposed development.

3.4.4. Water Link Services

In accordance with the GLS Revised Report, dated 19 April 2022, the following Link Water Mains will be required to accommodate the proposed development in the existing system (Reference to GLS Report dated 19 April 2022 – Annexure A : Figure 1) :

- > HStW1.2 : 880 m x 400 mm Ø new supply pipe to replace existing 150mm pipe
- > HStW1.3 : 305 m x 355 mm Ø new supply pipe

The incremental impact of the link services, for the development, on the relevant master plan items are as follows:

Table 3 : Incremental Impact of Development on relevant Master Plan Items		
ITEM	INCREMENTAL (%)	
HStW1.2 : 880 m x 400 mm Ø new supply pipe	7,97	
HStW1.3 : 305 m x 355 mm Ø new supply pip	8,73	

3.4.5. Water Reticulation

New 75/110/160mm class 12 MPVC water mains complete with isolating valves, fire hydrants and erf connections will be provided. Erf connections will be made with HDPE PE80 PN12,5 pipes and terminate with an end cap.



The basis of the water reticulation design for the proposed development is summarised in the table below:

Table 4 : Water Reticulation Design Criteria		
PARAMETER	GUIDELINE	
Pipe materials for erf connections	HDPE PE80 PN12,5	
Pipe materials for reticulation mains	MPVC (Class 12)	
Minimum diameter for reticulation mains	75mm	
Minimum diameter for erf connections serving two erven	25mm branching to 2 x 20mm	
Minimum diameter for erf connections serving one erf	20mm Polycop	
Valves	75/110/160mm AVK (open clockwise)	
Fire Hydrants	AVK London V on respective pipe Ø	
Water meters	20mm Elster Kent (Water meter to be installed by Hessequa Municipality with Building Plan approvals.)	

3.4.6. Bulk Supply Upgrading

The development of water sources is an ongoing exercise, driven by Hessequa Municipality, and funded through Capital Contributions payable per erf.

The development and incorporation of new boreholes, upgrading of bulk supply pipelines and pumping infrastructure will be in accordance with Hessequa Municipality's Master Planning. The developments impact, on existing bulk supply infrastructure, will be determined and confirmed by Hessequa Municipality. The development's incremental impact on the exploration of new water sources as well as the provision and or upgrading of new bulk infrastructure will be determined and negotiated with Hessequa Municipality.

3.5. SEWERAGE

3.5.1. WWTW

The existing WWTW consist over a design capacity of 4,0 Ml/d. The WWTW consists over sufficient capacity to accommodate the proposed development.



3.5.2. Sewer Design Flows

In accordance with the *Guidelines for the Provision of Engineering Services and Amenities in Residential Township Development* it is expected that 70% of the Average annual water daily demand will end up in the wastewater system.

The annual average dry weather flow (AADWF) equals 70% of 111,75 k ℓ /d = 78,23k ℓ /d = 0,905 ℓ /s.

To determine the Peak Wet Weather Flow (PWWF) a peak factor of 2,5 were taken in consideration with an expected stormwater infiltration of 15%. The PWWF equals 2,60 l/s.

3.5.3. Sewer Link and Bulk Infrastructure

The proposed development on Farm 591/1 drains to the south eastern boundary and falls within the existing Still Bay Adlepracht pumping station (PS) drainage area. A new link sewer main is required to connect the proposed development to the existing sewer infrastructure.

In accordance with the GLS Report the recommended position for a connection to the existing sewer system is to the 100 mm \emptyset gravity sewer in Melkboom Street. (Reference to GLS Report dated March 2022 – Annexure A : Figure 3) The existing sewer main between the point of connection and the existing 150 mm \emptyset outfall sewer in Main Road West has insufficient capacity to accommodate the additional sewage demand, from the development, and must be upgraded.

In accordance with the GLS Revised Report, dated 19 April 2022, the following link/upgrading of services items are required to connect the proposed development to the existing sewer system. (Reference to GLS Report dated March 2022 – Annexure A : Figures 3 & 4) :

- > HStS7.3 : 415m x 200mm Ø New outfall sewer
- > HStS7.4 : 210m x 200mm Ø Replace existing sewer
- > HStS7.5 : 120m x 200mm Ø New outfall sewer

It is further proposed that when the link service item HStS7.3 is constructed that the existing sewer system for the development directly to the north of Farm 591/1 be re-directed to the new outfall sewer, as shown on Figure 3 (Reference to GLS Report dated March 2022 – Annexure A). The gravity sewer system in Main Road West, which gravitates towards the Adlepracht PS, has sufficient spare capacity to accommodate the proposed development in the existing system.



The incremental impact of the link services, for the development, on the relevant master plan items are as follows:

Table 5 : Incremental Impact of Development on relevant Master Plan Items		
ITEM	INCREMENTAL (%)	
HStS7.3 : 415m x 200mm Ø New outfall sewer	100	
HStS7.4 : 210m x 200mm Ø Replace existing sewer	15,52	
HStS7.5 : 120m x 200mm Ø New outfall sewer	15,52	

3.5.4. Design Criteria

The following minimum design criteria shall be applicable to sewer pipework:

- Design parameters : Average daily flow as per Red Book for the different housing categories Peak factor – Harmon formula : Extraneous flow – 15% : Minimum velocity – 0.7m
- > Minimum cover to pipes : 0.80m
- Minimum pipe size : 110mm diameter for house connections : 160mm diameter for sewer mains
- Minimum gradients : 110mm diameter house connection 1:60 : main lines at 80% capacity as follows :

Number of Dwelling/units	Grade
Less than 6	1:80
6 to 10	1:100
11 to 80	1:120

- > House connection depth shall generally be 1.0m but at least be able to drain 80% of an erf.
- > Maximum manhole spacing of 80m.

3.6. ELECTRICAL SLEEVES

The position of communication and electrical sleeves (110/160mm Class 34 PVC) will be determined in consultation with the Electrical Engineer.



4. SOLID WASTE

Hessequa Municipality to collect solid waste for discharging at the Riversdale solid waste dump site. All garden refuge can be discharged at the Melkhoutfontein dump site.

The estimated solid waste generated per day, fully developed, is as follows:

3,5kg x person/d (784) = 2,744 ton/day = 2,058 m³/day (volume).

5. AUGMENTATION LEVIES

It is accepted that budgetary constraints, within the approved three-year Hessequa Municipality budget, will result in the Developer having to provide bridging finance for incremental services. The offset of Augmentation Levies, payable for water and sewer services, against the bridging finance of incremental services will be negotiated with Hessequa Municipality.

Levies, approved from 1 July 2022, are as follows:

General Residential Erven Zone II	: Water & Sewer : R 18,020.00/erf
General Residential Erven Zone IV	: Water & Sewer : R 7,155.00/erf
Business Zone III : 3,915 m ²	: Water & Sewer : R 10,759/1,000m ² (Building)

6. CONCLUSION

We trust that the information included in this report will provide insight to the level of services required for the development of Portion 1 of the Farm Duinekroon No 591.

G PEPLER Pr Tech Eng HESSEQUA CONSULTING ENGINEERS

29 June 2022



ANNEXURE A : DRAWINGS

- GLS FIGURE 1 : WATER LINK SERVICES
- GLS FIGURE 3 : SEWER LINK SERVICES
- GLS FIGURE 4 : SEWER LINK SERVICES









ANNEXURE B : DRAWINGS

- HESRIV-515-W1 : WATER TYPICAL DETAILS
- HESRIV-515-S1 : SEWER TYPICAL DETAILS









Traffic Impact Assessment

Proposed Residential Development, Stilbaai

Reference No. C1880 Prepared for Trevee Investments (Pty) Ltd 6 May 2022

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ABBREVIATIONS & SYMBOLS

ABBREVIATION / SYMBOL	DESCRIPTION
AMP	Arterial Management Plan
CBD	Central Business District
CITP	Current Integrated Transport Plan
СОТО	The Committee of Transport Officials South African
GLA	Gross Lettable Area
НА	Hectares
HCM	Highway Capacity Manual
LOS	Level of Service
LSDF	Local Spatial Development Framework
NMT	Non-Motorised Transport
SDF	Spatial Development Framework
SDP	Site Development Plan
SMEC	Snowy Mountains Engineering Corporation
SQM	Square Metres
STA	Site Traffic Assessment
SU+T	Single Unit plus Trailer
TIA	Traffic Impact Assessment
ТМН	Technical Methods for Highways
VPH	Vehicles Per Hour
WCG	Western Cape Government
~	Approximately Equal
±	Plus or Minus

1 Introduction

SMEC South Africa (Pty) Ltd was appointed by Trevee Investments (Pty) Ltd to conduct a Traffic Impact Assessment for the proposed residential development on Portion 1 of the farm Duinekroon No. 591, Stilbaai. The site is bounded by Buitekant Street to the north, and Thys Vissie Way to the south. A locality plan is shown in Figure 1-1.



Figure 1-1 Locality Plan

The site measures approximately 10 Hectares in extent and is planned to comprise of single residential units and townhouses. The proposed Site Layout Plan is shown in Figure 1-2.

The purpose of the Traffic Impact Assessment is to quantify the anticipated impact of the new development traffic. The study was conducted in accordance with The Committee of Transport Officials South African Traffic Impact and Site Traffic Assessment Manual (COTO, TMH 16 Volume 1).



Figure 1-2 Site Layout Plan
2 Background Information

2.1 Existing Roads

Figure 2-1 shows the existing roads the vicinity of the subject site.

Main Road provides connectivity between Stilbaai and the R305. Main Road is classified as a Class 3 Minor Arterial. In the vicinity of the site it comprises of a single carriageway road with one lane per direction, with dedicated right turning lanes provided at intersections. It experiences moderate traffic flows during peak hours.

Buitekant Street (between Main Road and Reservoir Street) is a Class 4 Distributor, serving the surrounding residential area. The road comprises of one lane per direction in the vicinity of the subject site. It experiences low traffic flows during peak hours.

Buitekant Street (west of Reservoir Street) is a Class 5 Local Street, serving the surrounding residential area. The road is an unsurfaced gravel road in the vicinity of the subject site. It experiences low traffic flows during peak hours.

Reservoir Street is a Class 5 Local Street, serving the surrounding residential area. The road comprises of one lane per direction in the vicinity of the subject site. It experiences low traffic flows during peak hours.



Figure 2-1 Existing Roads

2.2 Planned Roads

The Hessequa Local Municipality plans to extend Buitekant Street to the south (Road 1 in figure below) so that it bisects Thys Vissie Way and joins with Bosbokduin Avenue. Refer to Figure 2-2.



Figure 2-2 Planned Road Layout

2.3 Existing Public Transport Facilities

There are bus stops along Main Road within walking distance of 1 km to the subject site. Refer to Figure 2-3.



Figure 2-3 Existing Public Transport Facilities

2.4 Site Access

The subject site is proposed to be served by one public access, as follows (refer to Figure 2-5):

• Access 1: Along Road 1, ~105 metres south of Buitekant Street

The access spacing requirements were derived from the WCG Access Management Guidelines (2020).

According to the Hessequa SDF, the future Road 1 in the vicinity of the subject site will operate as a Class 5 Local Street. Refer to Figure 2-4. Furthermore, the roadside environment has been identified as a suburban area. It is therefore recommended that the access spacing requirements of a Class 5 Local Street in a suburban roadside environment be applied to Road 1.

According to the WCG Access Management Guidelines (2020), the anticipated volume of traffic that would use Access 1 of the development means that Access 1 would operate as an equivalent Class 4 Collector driveway.



Figure 2-4 Hessequa SDF Extract

The minimum spacing of an unsignalised full intersection along a Class 5 road within a suburban roadside environment, upstream and downstream of an unsignalised full intersection is 95 metres.

Access 1 would satisfy the minimum spacing requirements of 95 metres.



Figure 2-5 Proposed Site Access

3 Other Planning Authorities

Other than the Hessequa Local Municipality, no other planning authority would need to be included in the approval process.

4 Traffic Demand Estimation

4.1 Assessment Years

A base year assessment was undertaken to identify shortcomings in the road-based capacity in the short term, if any. In addition, thereto, it is required to grow traffic flows to an acceptable horizon year in order to ensure that the proposed road network would be able to operate satisfactorily once the development traffic is added to the surrounding road network.

TMH 16 Volume 1 Version 1.0, states that transportation improvements for developments must be designed for a horizon year of 5 years. Hence, a 2021 Base Year and a 2026 Design Year was used for this TIA.

4.2 Assessment Hours

The assessment has been undertaken considering the periods during which the combined effect of background and development traffic would result in the highest traffic demand. Hence, it was deemed suitable to assess the Weekday AM and PM Peak Hours.

4.3 Traffic Counts

Taking into consideration the location and extent of the proposed development with relation to the surrounding road network, the following traffic count surveys were undertaken as part of this project assignment:

- Counting Station 1: Intersection of Main Road and Buitekant Street;
- Counting Station 2: Intersection of Buitekant Street and Fynbos Street; and
- Counting Station 3: Intersection of Buitekant Street and Melkboom Street.

Traffic count locations are shown in Figure 4-1.



Figure 4-1 Traffic Count Locations

Details of the traffic survey are provided below:

- Date counted September 2021
 Day Wednesday during National State of Disaster Lockdown
- Level 3
- Congestion levels
 Low
- Enumerator Motion Consulting Engineers

The detailed traffic count data is provided in Appendix A.

A common peak hour was identified for the intersections under discussion, as follows:

- Weekday AM Peak Hour 08h00 09h00
- Weekday PM Peak Hour 15h45 16h45

The 2021 Base Year traffic flows are shown in Figure 4-2.

Traffic Demand Estimation

100 N (100)	Weeko	day AM day PM								(0) (0)	0	4 4 Reservoir Street	► 0 (0)		F 22 (24)	(1) (19) (4)	0 15 7		t 1 (3)	▲ 13 (26)	F 4 (5)	(0 (42)) 2) 4	0	+ + Pastorie Street	• 0		≠ 25 (41)	(70) (13)	58 7	+ 7	Buit	ekant	Street	(25) (164)	28 97	4 1 Excelsion Street	t 0 (0)		• 0 (0)	(71) (2) (91)	54 3 40	1 t t	▲ 68 (89)	← 216 (269)	F 4 (14)
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					(0)	0	*	(o)	0)							6	(17	(18	~	23	(36)								(42)		(119	÷	23	(30)							(86)	(272	(5)	ţ	2	(3)
		Bui	tekant S	Street	(0) 1 0 (0)	0 (0) 0	*	1	•									Melkboom Street													Fynbos Street												Main Road			
Road 3		(0) •	0) 0 F	Road 2	(0)	0	•	▲ 0 (0)	(0) •																																					
1 1 0 0 (0)		0	(0)		1 0 (0)	↑ 0 (0)		Road 1																																						

Figure 4-2 2021 Base Year Traffic Flows

4.4 Traffic Growth Rates

A traffic growth rate is applied to background traffic in order to determine the anticipated growth in this traffic besides that relating to planned and new developments. The Committee of Transport Officials Trip Data Manual (COTO, TMH 17 Volume 1 Version 1.01) provides typical growth rates to be used for growth areas based on the existing/anticipated rate of growth. Refer to Table 4-1.

Development Area	Growth Rate
Low Growth Areas	0% - 3%
Average Growth Areas	3% - 4%
Above Average Growth Areas	4% - 6%
Fast Growing Ares	6% - 8%
Exceptionally High Growth Areas	> 8%

Table 4-1 Typical Growth Rates

Taking into consideration the nature and extent of development within this area, an annual compounded traffic growth rate of 1.0% was applied to the 2021 background traffic along Main Road in order to derive the 2026 Design Year traffic flows.

The 2026 Forecast Year traffic flows are shown in Figure 4-3.

Traffic Demand Estimation

100 Weekday AM N (100) Weekday PM (0)		(c)	teau teau <thteau< th=""> teau teau <th< th=""></th<></thteau<>
	• 19 (33) • 1	• 2 (7) • 9 (27) •	r a 89 (99) f f a 10 (6)
Q Q 4 0 (0)	(1) (1) (2) - (0) 0 → (0) 2 →	← 11 (23) ← 36 (66) 두 ✔ 23 (36) ♀	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
Buitekant Street (0) 0	Mel kboom Street		Fynbos Street Main Road
read 0 0 0 1 0 0 1 0 0 1 0 0 1 1 0 0 1			

Figure 4-3 2026 Forecast Year Traffic Flows

5 Trip Generation, Distribution and Traffic Assignment

5.1 Trip Generation

The Trip Generation Rates for the land use types forming part of the development were obtained from the COTO TMH 17 South African Trip Data Manual dated September 2012.

The trip generation potential of the development is shown in Table 5-1.

Table	5-1	Trin	Generation
Iable	J-1	mp	Oeneration

			Di	rectiona	l Split (%)	Т	ips gene	rated (vp	h)
Land Use	Dwelling units (du)	Trip rate (trips	Wee A	kday M	Wee P	kday M	Weekd	lay AM	Weeko	lay PM
		/du)	IN	OUT	IN	OUT	IN	OUT	IN	OUT
Residential – Single Dwelling Unit	146	1	25%	75%	70%	30%	37	110	102	44
Residential – Townhouses	32	0.85	25%	75%	70%	30%	7	20	19	6
ALL New Trips							17	4	17	'3

It is anticipated that the proposed development would generate 174 and 173 new vehicular trips during the Weekday AM and PM Peak Hours, respectively.

5.2 Trip Distribution

Trip distribution was estimated manually, based on existing traffic flows, the land use of the surrounding areas and the proposed access configuration. Refer to Figure 5-1.

5.3 Traffic Assignment

Traffic assignment involves determining the amount of traffic that will use specific routes in the network. The traffic assignment is made with consideration to logical routings, available roadway capacity, right-turn movements, travel times and other factors. Refer to Figure 5-2.

The 2021 Base + Development Trips are shown in Figure 5-3.

The 2026 Base + Development Trips are shown in Figure 5-4.



Figure 5-1 Trip Distribution (New Trips)

100 W N (100) W	/eekday AM /eekday PM						Reservoir Street												Pastorie Street								Excelsior Street								
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	Buitekant Street	(0) 1 0 (0)	0 7 (25) 130	۰ ۱	44								Melkboom Street											Fynbos Street								Main Road			
Road 3	(⊧ɛ) /∠ 0 ↓ • Road 2	(52) 1	30 🛥 0 🗣	t 44 (121)	(0) ↓																l Access 2														
(0) 0 + (13) 33 •	 ⊷ 26 (71) ೯ 11 (31) 	1 0 (0)	t D (D)	Road 1																	Mall														

Figure 5-2 Traffic Assignment (New Trips)

100 V N (100) V	Neekday AM Neekday PM					((0) (52)	0 130	4 4 Reservoir Street	▲ 0 (0)		🛩 22 (24)	(1) (71 (4)) 0) 14) -	•	t 1 (3) t 13 (26)			(0) (94)	0 170	↓ Is Pastorie Street	▲ 0 (0)		F 25 (41)	(122) (13)	188 7	+ 7	Buitek	ant Street	(25) (216)	28	• 0 (0)	0		(94) (2) (120)	129 3 95	* + *	▲ 89 (151)	+ 216 (269)	4 (14)
	Buitekant Stre	et	(0) (0) 1 0 (0)	(52) 130 † 0 0	0	(17) #				~ ~	19 44	(33) (121)	ኅ 2 (2)	(12) 24	Melkhoom Street (18) 21 J		► 2 ► 55 ► 23	(5 (1 3 (;	(7) 144) (36)				•	9 80	(27) (187)	(42) 11 🕈		Fynbos Street (119) 67 🤜	← 7 ☞ 2	8 (172) 3 (30)			▲ 8 ← 10	9 (99	9) (2)	(145) 98 🚽	(272) 244 🕈	Main Road (5) 2 🥊	~ ·	10 (6 3 (5 2 (3	3) 5) 3)
(0) 0 + Road 3	0 (E) 0 E ↓ 4 ↓ 4 ★ 26 ↓ 11 (31)	(1d 2	52) (0) 1 0 (0)	130	Boad 1 4- 44 (121)											•																									

Figure 5-3 2021 Base + Development Trips

100 Weekday AM N (100) Weekday PM	Pastorie Street	Excelsion Street
(0) 0 -		(25) 28 ▲ (25) <td< th=""></td<>
(52) 130 🏲	○ S (71) 145	
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	← 44 (121) ト & & + 55 (144)	$\leftarrow 80 (187) \qquad \qquad$
<i>1</i> , 3, 3, 4 0 (0)	€ ₽ ₽ 23 (36)	$\widehat{\mathbf{Q}} = \begin{bmatrix} \mathbf{r} & 23 & (30) \end{bmatrix} = \begin{bmatrix} \mathbf{r} & 23 & (30) \end{bmatrix} = \begin{bmatrix} \mathbf{r} & 23 & (30) \end{bmatrix} = \begin{bmatrix} \mathbf{r} & 23 & (30) \end{bmatrix}$
Buitekant Street (0) 0 → ↓ 1 1 1 1 0 82 0 32 0 12 32 1	Melikboom Street	Fynbos Street Main Road
Freeze (0) (52) 130 - (15) (0) 0 (5) (52) 130 - + + 0 - - - - - + <t< th=""><th></th><th>Mall Access 2</th></t<>		Mall Access 2

Figure 5-4 2026 Base + Development Trips

6 Traffic Analysis

Intersection capacity analyses were undertaken to determine the anticipated operational performance of the site accesses and surrounding road network, taking into consideration the phased implementation of the development and associated development trips. The state-of-the-art traffic engineering software package, SIDRA Intersection 8.0 software, was used. The intersections analysed for the development are listed below:

- Main Road and Buitekant Street;
- Buitekant Street and Excelsior Street;
- Buitekant Street and Fynbos Street;
- Buitekant Street and Pastorie Street;
- Buitekant Street and Melkboom Street;
- Buitekant Street and Reservoir Street;
- Buitekant Street and Road 1;
- Road 1 and Road 2; and
- Road 2 and Road 3.

The intersection of Buitekant Street and Road 3 was not analysed as part of this exercise as it was assumed that all the development trips would use Access 1 during the Weekday AM and PM Peak Hours.

The following scenarios were analysed as part of this project assignment:

- 2021 Background Traffic;
- 2021 Background + Development Traffic; and
- 2026 Background + Development Traffic.

The operational performance of an intersection is typically quantified in terms of Level of Service as defined by the US Highway Capacity Manual (HCM). These definitions relate average delays at intersections (for individual turning movements, for each approach and for the overall intersection) to a level of service ranging from A to F, as are shown in Table 6-1.

Level of Service	Control Delay per V	ehicle in Seconds (d)	LOS for V/C Ratio
	Signals and Roundabouts	Stop Signs and Yield Signs	V/C > 1
Α	d ≤ 10	d ≤ 10	F
В	10 <d 20<="" th="" ≤=""><th>10 <d 15<="" th="" ≤=""><th>F</th></d></th></d>	10 <d 15<="" th="" ≤=""><th>F</th></d>	F
С	20 < d ≤ 35	15 < d ≤ 25	F
D	35 < d ≤ 55	25 < d ≤ 35	F
E	55 < d ≤ 80	35 < d ≤ 50	F
F	80 < d	50 < d	F

Table 6-1: Intersection-Based Level of Service Criteria

The following sub-sections set out the intersection capacity analyses of the proposed development.

Detailed Sidra outputs are contained in Appendix B.

6.1 Main Road and Buitekant Street

The existing intersection of Main Road and Buitekant Street is a priority-controlled intersection with stop controls along each approach. The north approach comprises of a shared through-and-left lane and short right-turn lane. The east approach comprises of a single lane serving all movements. The south approach comprises of a shared through-and-left lane and short right-turn lane. The west approach comprises of a single lane serving all movements. Refer to Figure 6-1.



Figure 6-1 Main Road and Buitekant Street: Existing Layout

2021 Background Traffic

Taking into consideration the 2021 Base Year traffic flows, the intersection is currently operating at Level of Service F and E during the Weekday AM and PM Peak Hours, with an average delay of approximately 59 and 46 seconds respectively.

It is concluded that the existing intersection configuration is unable to accommodate the 2021 Background Traffic at an acceptable Level of Service.

2021 Background + Development Traffic – Existing Layout

Taking into consideration the existing intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at Level of Service F during both the Weekday AM and PM Peak Hours, with an average delay of approximately 73 and 80 seconds respectively.

It is concluded that the existing intersection configuration would be unable to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

In order to improve operations and minimise delay along Main Road, the proposed intersection of Main Road and Buitekant Street would be converted to a roundabout with two circulating lanes. The north approach would comprise of a shared through-and-left lane and a short shared through-and-right lane. The east approach would comprise of a single lane serving all movements. The south approach would comprise of a shared through-and-left lane and a short shared through-and-right lane. The west approach would comprise of a single lane serving all movements. Refer to Figure 6-2.



Figure 6-2 Main Road and Buitekant Street: Proposed Layout

2021 Background + Development Traffic – Proposed Layout

Taking into consideration the proposed intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 6 seconds.

It is concluded that the proposed intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Proposed Layout

Taking into consideration the proposed intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 6 seconds.

6.2 Buitekant Street and Excelsior Street

The existing intersection of Buitekant Street and Excelsior Street is a priority-controlled T-junction with a stop control along the north approach. All approaches comprise of one lane serving all movements. Refer to Figure 6-3.



Figure 6-3 Buitekant Street and Excelsior Street: Existing Layout

2021 Background Traffic

Taking into consideration the 2021 Base Year traffic flows, the priority-controlled side road is currently operating at Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 9 seconds.

It is concluded that the existing intersection configuration is able to accommodate the 2021 Background Traffic at an acceptable Level of Service.

2021 Background + Development Traffic - Existing Layout

Taking into consideration the existing intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the priority-controlled side road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 9 and 10 seconds respectively.

It is concluded that the existing intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Existing Layout

Taking into consideration the existing intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the priority-controlled side road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 9 and 10 seconds respectively.

6.3 Buitekant Street and Fynbos Street

The existing intersection of Buitekant Street and Fynbos Street is a priority-controlled T-junction with a stop controls along all approaches. All approaches comprise of one lane serving all movements. Refer to Figure 6-4.



Figure 6-4 Buitekant Street and Fynbos Street: Existing Layout

2021 Background Traffic

Taking into consideration the 2021 Background Traffic flows, the intersection is currently operating at an overall Level of Service B and C during the Weekday AM and PM Peak Hours, with an average delay of approximately 15 seconds.

It is concluded that the existing intersection configuration is able to accommodate the 2021 Background Traffic at an acceptable Level of Service.

2021 Background + Development Traffic - Existing Layout

Taking into consideration the existing intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service B and C during the Weekday AM and PM Peak Hours, with an average delay of approximately 15 and 16 seconds respectively.

It is concluded that the existing intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Existing Layout

Taking into consideration the existing intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service B and C during the Weekday AM and PM Peak Hours, with an average delay of approximately 15 and 16 seconds respectively.

6.4 Buitekant Street and Pastorie Street

The existing intersection of Buitekant Street and Pastorie Street is a priority-controlled T-junction with a stop control along the north approach. All approaches comprise of one lane serving all movements. Refer to Figure 6-5.



Figure 6-5 Buitekant Street and Pastorie Street: Existing Layout

2021 Background Traffic

Taking into consideration the 2021 Base Year traffic flows, the priority-controlled side road is currently operating at Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 8 seconds.

It is concluded that the existing intersection configuration is able to accommodate the 2021 Background Traffic at an acceptable Level of Service.

2021 Background + Development Traffic – Existing Layout

Taking into consideration the existing intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the priority-controlled side road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 9 and 8 seconds respectively.

It is concluded that the existing intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Existing Layout

Taking into consideration the existing intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the priority-controlled side road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 9 and 8 seconds respectively.

6.5 Buitekant Street and Melkboom Street

The existing intersection of Buitekant Street and Melkboom Street is a priority-controlled intersection with stop controls along all approaches. All approaches comprise of one lane serving all movements. Refer to Figure 6-6.



Figure 6-6 Buitekant Street and Melkboom Street: Existing Layout

2021 Background Traffic

Taking into consideration the 2021 Base Year traffic flows, the intersection is currently operating at Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 13 seconds.

It is concluded that the existing intersection configuration is able to accommodate the 2021 Background Traffic at an acceptable Level of Service.

2021 Background + Development Traffic – Existing Layout

Taking into consideration the existing intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 14 seconds.

It is concluded that the existing intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Existing Layout

Taking into consideration the existing intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 14 seconds.

6.6 Buitekant Street and Reservoir Street

The existing intersection of Buitekant Street and Reservoir Street is a priority-controlled T-junction with stop controls along all approaches. All approaches comprise of one lane serving all movements. Refer to Figure 6-7.



Figure 6-7 Buitekant Street and Reservoir Street: Existing Layout

2021 Background Traffic

Taking into consideration the 2021 Base Year traffic flows, the intersection is currently operating at Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 12 and 13 seconds respectively.

It is concluded that the existing intersection configuration is able to accommodate the 2021 Background Traffic at an acceptable Level of Service.

2021 Background + Development Traffic - Existing Layout

Taking into consideration existing intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service C during both the Weekday AM and PM Peak Hours, with an average delay of approximately 20 and 21 seconds respectively.

It is concluded that the existing intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Existing Layout

Taking into consideration existing intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the intersection is anticipated to operate at an overall Level of Service C during both the Weekday AM and PM Peak Hours, with an average delay of approximately 20 and 21 seconds respectively.

6.7 Buitekant Street and Road 1

The proposed intersection of Buitekant Street and Road 1 would be a priority-controlled T-junction with a stop control along the west approach. All approaches would comprise of one lane serving all movements. Refer to Figure 6-8.



Figure 6-8 Buitekant Street and Road 1: Proposed Layout

2021 Background + Development Traffic

Taking into consideration the proposed intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the priority-controlled access road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 8 seconds.

It is concluded that the proposed intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Proposed Layout

Taking into consideration the proposed intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the priority-controlled access road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 8 seconds.

6.8 Road 1 and Road 2

The proposed intersection of Road 1 and Road 2 would be a priority-controlled T-junction with a stop control along the west approach. All approaches would comprise of one lane serving all movements. Refer to Figure 6-9Figure 6-8.



Figure 6-9 Road 1 and Road 2: Proposed Layout

2021 Background + Development Traffic

Taking into consideration the proposed intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the priority-controlled access road is anticipated to operate at an overall Level of Service B and C during the Weekday AM and PM Peak Hours, with an average delay of approximately 14 and 22 seconds respectively.

It is concluded that the proposed intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Proposed Layout

Taking into consideration the proposed intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the priority-controlled access road is anticipated to operate at an overall Level of Service B and C during the Weekday AM and PM Peak Hours, with an average delay of approximately 14 and 22 seconds respectively.

6.9 Road 2 and Road 3

The proposed intersection of Road 2 and Road 3 would be a priority-controlled T-junction with a stop control along the east approach. All approaches would comprise of one lane serving all movements. Refer to Figure 6-10.



Figure 6-10 Road 2 and Road 3: Proposed Layout

2021 Background + Development Traffic

Taking into consideration the proposed intersection layout as well as the 2021 Background Traffic plus the anticipated development traffic flows, the priority-controlled side road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 8 seconds.

It is concluded that the proposed intersection configuration would be able to accommodate the 2021 Background plus Development Traffic at an acceptable Level of Service.

2026 Background + Development Traffic – Proposed Layout

Taking into consideration the proposed intersection layout as well as the 2026 Background Traffic plus the anticipated development traffic flows, the priority-controlled side road is anticipated to operate at an overall Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 8 seconds.

7 Other Road Users

7.1 Public Transport

There are several public transport lay-byes provided along Main Road, which are within walking distance of subject site. Refer to Figure 7-1.



Figure 7-1 Existing Public Transport Facilities

It is our submission that the existing public transport lay-byes along Main Road are adequate to serve the proposed development, therefore no further provision need to be made for public transport.

7.2 Non-Motorised Transport

No provision is made for pedestrian sidewalks along Class 4 or 5 Roads within Stilbaai. It is therefore our submission that no provision would need to be made for pedestrian movements as part of the proposed development.

8 Site Impact Assessment

This report does not include a Site Traffic Assessment, as the building plans are not yet finalised. Reference is however made to general design criteria to be considered in compilation of the site development plan.

8.1 Internal Operations

Provision should be made for ease of circulation as part of the internal layout of the planned development. Security-controlled access could be provided to the development for the safety of the residents, as long as the corresponding throat length and queue length requirements are satisfied.

8.2 Throat Length

Adequate throat length provision is essential in ensuring sufficient operation of a development access and preventing possible spill-back onto the surrounding public road.

The WCG Access Management Guidelines (2020) sets out minimum requirements pertaining to access throat lengths for driveways serving private commercial or residential developments. Based on this, the following are required:

• A minimum ingress and egress throat length of 20 metres is required along a Class 4 or lower order road.

The Site Development Plan would need to make suitable provision for throat lengths for both the ingress and egress movements at the development access.

8.3 Parking

Parking provision is an important consideration of any development and would ultimately ensure that vehicular traffic is accommodated on-site in its entirety. Insufficient parking provision would have a negative impact on the operational performance of the site and surrounding public roads, as well as on road safety.

The Hessequa Zoning Scheme By-Law was used to ascertain the parking standards to be adhered to, based on the site-specific land use rights, as follows. For normal areas:

- Erven > 350 m², a minimum of 2 parking bays would be required per dwelling unit.
- 100 m² < Erven < 350 m², a minimum of 1 parking bay would be required per dwelling unit.
- Erven < 100 m², zero parking bays would be required per dwelling unit.

8.4 Loading

The proposed design vehicle would be a Single Unit (SU) truck, with a turning radius of approximately 10 metres. The turning path of the design vehicle would need to be tested to ensure that the site development plan would be sufficient to accommodate truck turning movements on site to allow refuse collection.

9 Proposed Improvements

The following road capacity improvements are proposed:

2021 Base Year:

Convert the intersection of Main Road and Buitekant Street to a roundabout with two circulating lanes. The north approach would comprise of a shared through-and-left lane plus a short shared through-and-right lane. The east approach would comprise of a single lane serving all movements. The south approach would comprise of a shared through-and-left lane plus a short shared through-and-right lane. The west approach would comprise of a single lane serving all movements.

10 Conclusion and Recommendations

- SMEC South Africa (Pty) Ltd was appointed by Trevee Investments (Pty) Ltd to conduct a Traffic Impact Assessment for the proposed residential development on Portion 1 of the farm Duinekroon No. 591, Stilbaai. The site is bounded by Buitekant Street to the north, and Thys Vissie Way to the south.
- \circ ~ The subject site is proposed to be served by one public access, as follows:
 - Access 1: Along Road 1, ~105 metres south of Buitekant Street
- It is anticipated that the proposed development would generate 174 and 173 new vehicular trips during the Weekday AM and PM Peak Hours, respectively.
- It is our submission that the existing bus stops along Main Road are adequate to serve the proposed development, therefore no further provision would need to be made for public transport.
- The following road capacity improvements are proposed:

2021 Base Year:

Convert the intersection of Main Road and Buitekant Street to a roundabout with two circulating lanes. The north approach would comprise of a shared through-and-left lane plus a short shared through-and-right lane. The east approach would comprise of a single lane serving all movements. The south approach would comprise of a shared through-and-left lane plus a short shared through-and-right lane. The west approach would comprise of a single lane serving all movements.

This Traffic Impact Assessment is supported from a traffic engineering perspective, provided that the required improvements be implemented in accordance with appropriate design standards.

Appendices

Appendix A – Detailed Traffic Counts Appendix B – Detailed SIDRA Outputs

Appendix A Detailed Traffic Counts

Main Road and Buitekant Street





15 min inter	survey rvals	Total 15min	Total	Peak	Bus	Heavy	Taxi			1					2					3					4					5		
Start	End	Interval Volumes	Volumes	Factor	0.8%	2.2%	3.2%	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light
06:00	06:15	33						1	0	0	0	1	0	0	0	0	0	1	0	0	0	1	2	0	0	0	2	14	0	0	0	14
06:15	06:30	40						3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	18	3	0	3	12
06:30	06:45	53						7	0	0	0	7	0	0	0	0	0	3	0	0	0	3	1	0	0	0	1	27	0	0	1	26
06:45	07:00	78	204	0.65	2.6%	3.8%	7.7%	4	0	0	0	4	0	0	0	0	0	2	0	0	1	1	1	0	0	0	1	29	1	1	2	25
07:00	07:15	137	308	0.56	2.9%	1.5%	5.8%	11	0	0	2	9	0	0	0	0	0	7	0	0	0	7	7	0	0	0	7	56	2	0	2	52
07:15	07:30	191	459	0.6	0.5%	1.0%	2.6%	13	0	0	0	13	2	0	0	0	2	8	0	1	0	7	4	0	0	0	4	66	1	1	2	62
07:30	07:45	167	573	0.75	1.8%	0.6%	4.2%	11	0	0	0	11	0	0	0	0	0	5	0	0	0	5	6	0	0	0	6	53	1	1	2	49
07:45	08:00	191	686	0.9	0.5%	2.1%	6.3%	9	0	0	1	8	3	0	0	0	3	9	0	0	0	9	9	0	0	1	8	72	1	0	5	66
08:00	08:15	182	731	0.96	0.0%	4.9%	3.3%	11	0	1	1	9	2	0	0	0	2	10	0	0	0	10	21	0	0	0	21	63	0	5	2	56
08:15	08:30	193	733	0.95	0.0%	5.2%	2.1%	13	0	0	0	13	1	0	0	0	1	14	0	0	0	14	14	0	0	0	14	73	0	5	2	66
08:30	08:45	175	741	0.96	0.0%	3.4%	1.1%	13	0	0	0	13	0	0	0	0	0	6	0	0	0	6	22	0	0	0	22	61	0	4	1	56
08:45	09:00	171	721	0.93	1.2%	4.7%	3.5%	17	0	0	1	16	0	0	0	0	0	10	0	0	1	9	18	0	1	0	17	47	1	2	2	42
15:00	15:15	216						36	0	3	1	32	0	0	0	0	0	20	0	1	0	19	26	0	1	0	25	49	1	1	2	45
15:15	15:30	201						18	0	0	0	18	1	0	0	0	1	19	0	0	0	19	21	0	3	1	17	67	0	3	1	63
15:30	15:45	193						25	0	1	0	24	0	0	0	0	0	21	0	0	2	19	17	0	0	0	17	59	0	3	2	54
15:45	16:00	224	834	0.93	0.9%	2.2%	1.8%	11	0	1	0	10	1	0	0	0	1	24	0	0	2	22	21	0	0	0	21	64	1	0	2	61
16:00	16:15	252	870	0.86	0.8%	3.2%	1.6%	20	0	0	0	20	0	0	0	0	0	19	0	0	1	18	24	0	0	0	24	68	2	6	1	59
16:15	16:30	216	885	0.88	0.0%	0.9%	2.8%	15	0	0	0	15	0	0	0	0	0	27	0	0	2	25	25	0	0	0	25	63	0	0	2	61
16:30	16:45	221	913	0.91	0.5%	0.5%	2.3%	25	0	0	1	24	1	0	0	0	1	21	0	0	1	20	16	0	0	0	16	77	0	1	1	75
16:45	17:00	212	901	0.89	0.9%	0.9%	3.3%	25	0	0	2	23	2	0	0	0	2	10	0	0	0	10	22	0	0	0	22	65	1	1	3	60
17:00	17:15	215	864	0.98	0.5%	1.4%	1.9%	29	0	0	1	28	0	0	0	0	0	13	0	0	1	12	20	0	0	0	20	59	1	3	2	53
17:15	17:30	167	815	0.92	0.6%	1.2%	2.4%	21	0	0	1	20	1	0	0	0	1	18	0	0	0	18	13	0	0	0	13	52	1	2	1	48
17:30	17:45	149	743	0.86	0.7%	1.3%	1.3%	20	0	0	0	20	0	0	0	0	0	18	0	0	0	18	12	0	0	0	12	34	0	2	0	32
17:45	18:00	104	635	0.74	0.0%	0.0%	2.9%	12	0	0	0	12	0	0	0	0	0	12	0	0	1	11	3	0	0	0	3	28	0	0	2	26

15 min inte	survey vals			6					7					8					9					10					11					12		
Start	End	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light
06:00	06:15	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	0	11	2	0	0	0	2
06:15	06:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16	3	0	5	8	0	0	0	0	0
06:30	06:45	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2	0	0	1	1	1	0	0	1	0	6	1	0	2	3	4	0	0	1	3
06:45	07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	39	1	2	3	33	3	0	0	0	3
07:00	07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	49	2	2	3	42	6	0	0	1	5
07:15	07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	89	0	0	3	86	8	0	0	0	8
07:30	07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	74	2	0	5	67	18	0	0	0	18
07:45	08:00	3	0	0	0	3	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	3	0	0	0	3	69	0	3	4	62	12	0	1	1	10
08:00	08:15	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	2	0	0	0	2	57	0	3	3	51	14	0	0	0	14
08:15	08:30	2	0	0	0	2	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	62	0	5	2	55	12	0	0	0	12
08:30	08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	1	0	0	0	1	46	0	2	1	43	22	0	0	0	22
08:45	09:00	0	0	0	0	0	1	0	0	0	1	1	0	0	0	1	5	0	0	0	5	1	0	0	0	1	51	1	4	1	45	20	0	1	1	18
15:00	15:15	1	0	0	0	1	2	0	0	0	2	0	0	0	0	0	1	0	0	0	1	1	0	0	0	1	60	0	0	3	57	20	0	0	0	20
15:15	15:30	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	46	1	2	1	42	26	0	1	0	25
15:30	15:45	2	0	0	0	2	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	49	0	1	1	47	19	0	0	1	18
15:45	16:00	3	0	0	0	3	1	0	0	0	1	2	0	0	0	2	1	0	0	0	1	5	0	0	0	5	76	1	4	0	71	15	0	0	0	15
16:00	16:15	0	0	0	0	0	1	0	0	0	1	3	0	0	0	3	3	0	0	0	3	4	0	0	0	4	84	0	2	2	80	26	0	0	0	26
16:15	16:30	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1	3	0	0	0	3	55	0	2	2	51	25	0	0	0	25
16:30	16:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	2	0	0	0	2	54	1	0	1	52	23	0	0	1	22
16:45	17:00	3	0	0	0	3	2	0	0	0	2	0	0	0	0	0	2	0	0	0	2	1	0	0	0	1	61	1	1	2	57	19	0	0	0	19
17:00	17:15	1	0	0	0	1	3	0	0	0	3	2	0	0	0	2	5	0	0	0	5	1	0	0	0	1	53	0	0	0	53	29	0	0	0	29
17:15	17:30	1	0	0	0	1	1	0	0	0	1	1	0	0	0	1	2	0	0	0	2	2	0	0	0	2	38	0	0	1	37	17	0	0	1	16
17:30	17:45	3	0	0	0	3	3	0	0	1	2	2	0	0	0	2	1	0	0	0	1	3	0	0	1	2	36	1	0	0	35	17	0	0	0	17
17:45	18:00	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	0	0	0	0	0	35	0	0	0	35	9	0	0	0	9






15 min inter	survey rvals	Total 15min	Total	Peak	Bus	Heavy	Taxi			1					2					3					4					5		
Start	End	Interval Volumes	Volumes	Factor	0.0%	0.7%	0.1%	Total	Bus	Heavy	Ta xi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Ta xi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light
06:00	06:15	3											1	0	0	0	1	0	0	0	0	0	0	0	0	0	0					
06:15	06:30	3											1	0	0	0	1	0	0	0	0	0	0	0	0	0	0					
06:30	06:45	12											5	0	0	0	5	0	0	0	0	0	0	0	0	0	0					
06:45	07:00	13	31	0.6	0.0%	0.0%	0.0%						4	0	0	0	4	0	0	0	0	0	0	0	0	0	0					
07:00	07:15	33	61	0.46	0.0%	0.0%	0.0%						16	0	0	0	16	3	0	0	0	3	2	0	0	0	2					
07:15	07:30	31	89	0.67	0.0%	3.2%	0.0%						16	0	1	0	15	1	0	0	0	1	1	0	0	0	1					
07:30	07:45	31	108	0.82	0.0%	0.0%	0.0%						8	0	0	0	8	3	0	0	0	3	1	0	0	0	1					
07:45	08:00	33	128	0.97	0.0%	3.0%	0.0%						17	0	0	0	17	0	0	0	0	0	2	0	0	0	2					
08:00	08:15	50	145	0.73	0.0%	2.0%	0.0%						14	0	0	0	14	3	0	0	0	3	1	0	0	0	1					
08:15	08:30	56	170	0.76	0.0%	0.0%	0.0%						18	0	0	0	18	2	0	0	0	2	4	0	0	0	4					
08:30	08:45	40	179	0.8	0.0%	0.0%	0.0%						8	0	0	0	8	1	0	0	0	1	3	0	0	0	3					
08:45	09:00	54	200	0.89	0.0%	3.7%	0.0%						18	0	0	0	18	1	0	0	0	1	3	0	0	0	3					
15:00	15:15	76											13	0	1	0	12	5	0	0	0	5	10	0	1	0	9					
15:15	15:30	95											13	0	1	0	12	6	0	0	0	6	9	0	0	0	9					
15:30	15:45	77											14	0	0	0	14	0	0	0	0	0	9	0	0	0	9					
15:45	16:00	79	327	0.86	0.0%	1.3%	1.3%						23	0	1	0	22	2	0	0	0	2	9	0	0	0	9					
16:00	16:15	86	337	0.89	0.0%	0.0%	0.0%						10	0	0	0	10	6	0	0	0	6	17	0	0	0	17					
16:15	16:30	82	324	0.94	0.0%	0.0%	0.0%						15	0	0	0	15	2	0	0	0	2	13	0	0	0	13					
16:30	16:45	78	325	0.94	0.0%	0.0%	0.0%						22	0	0	0	22	3	0	0	0	3	3	0	0	0	3					
16:45	17:00	78	324	0.94	0.0%	0.0%	1.3%						14	0	0	0	14	6	0	0	0	6	8	0	0	0	8					
17:00	17:15	81	319	0.97	0.0%	0.0%	0.0%						14	0	0	0	14	2	0	0	0	2	9	0	0	0	9					
17:15	17:30	62	299	0.92	0.0%	0.0%	0.0%						15	0	0	0	15	4	0	0	0	4	11	0	0	0	11					
17:30	17:45	61	282	0.87	0.0%	0.0%	0.0%						8	0	0	0	8	1	0	0	0	1	10	0	0	0	10					
17:45	18:00	38	242	0.75	0.0%	0.0%	0.0%						7	0	0	0	7	0	0	0	0	0	6	0	0	0	6					

15 min inte	survey rvals			6					7					8					9					10					11					12		
Start	End	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light
06:00	06:15	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1																				
06:15	06:30	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0																				
06:30	06:45	3	0	0	0	3	2	0	0	0	2	2	0	0	0	2																				
06:45	07:00	3	0	0	0	3	1	0	0	0	1	5	0	0	0	5																				1
07:00	07:15	3	0	0	0	3	3	0	0	0	3	6	0	0	0	6																				
07:15	07:30	5	0	0	0	5	2	0	0	0	2	6	0	0	0	6																				
07:30	07:45	8	0	0	0	8	5	0	0	0	5	6	0	0	0	6																				
07:45	08:00	6	0	0	0	6	5	0	0	0	5	3	0	1	0	2																				
08:00	08:15	12	0	1	0	11	5	0	0	0	5	15	0	0	0	15																				
08:15	08:30	19	0	0	0	19	4	0	0	0	4	9	0	0	0	9																				
08:30	08:45	16	0	0	0	16	7	0	0	0	7	5	0	0	0	5																				
08:45	09:00	20	0	0	0	20	7	0	2	0	5	5	0	0	0	5																				1
15:00	15:15	34	0	1	0	33	5	0	1	0	4	9	0	0	0	9																			1	
15:15	15:30	50	0	1	0	49	8	0	0	0	8	9	0	3	0	6																			1	I
15:30	15:45	39	0	1	0	38	7	0	0	1	6	8	0	0	0	8																				L
15:45	16:00	28	0	0	1	27	8	0	0	0	8	9	0	0	0	9																				
16:00	16:15	29	0	0	0	29	8	0	0	0	8	16	0	0	0	16																				<u> </u>
16:15	16:30	33	0	0	0	33	8	0	0	0	8	11	0	0	0	11																				L
16:30	16:45	29	0	0	0	29	6	0	0	0	6	15	0	0	0	15																				<u> </u>
16:45	17:00	29	0	0	1	28	5	0	0	0	5	16	0	0	0	16																				<u> </u>
17:00	17:15	35	0	0	0	35	6	0	0	0	6	15	0	0	0	15																				ļ
17:15	17:30	22	0	0	0	22	2	0	0	0	2	8	0	0	0	8																				
17:30	17:45	33	0	0	0	33	4	0	0	0	4	5	0	0	0	5																				<u> </u>
17:45	18:00	21	0	0	0	21	0	0	0	0	0	4	0	0	0	4																				







15 min inte	survey rvals	Total 15min	Total	Peak	Bus	Heavy	Тахі			1					2					3					4					5		
Start	End	Interval Volumes	Volumes	Hour Factor	0.0%	1.4%	0.0%	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light
06:00	06:15	1						0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15	06:30	1						0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:30	06:45	3						0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45	07:00	13	18	0.35	0.0%	0.0%	0.0%	0	0	0	0	0	3	0	0	0	3	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0
07:00	07:15	15	32	0.53	0.0%	0.0%	0.0%	0	0	0	0	0	3	0	0	0	3	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1
07:15	07:30	27	58	0.54	0.0%	3.7%	0.0%	0	0	0	0	0	5	0	1	0	4	2	0	0	0	2	0	0	0	0	0	1	0	0	0	1
07:30	07:45	13	68	0.63	0.0%	0.0%	0.0%	0	0	0	0	0	3	0	0	0	3	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1
07:45	08:00	19	74	0.69	0.0%	10.5%	0.0%	0	0	0	0	0	4	0	0	0	4	1	0	0	0	1	1	0	0	0	1	5	0	1	0	4
08:00	08:15	36	95	0.66	0.0%	0.0%	0.0%	0	0	0	0	0	5	0	0	0	5	3	0	0	0	3	0	0	0	0	0	8	0	0	0	8
08:15	08:30	32	100	0.69	0.0%	0.0%	0.0%	0	0	0	0	0	3	0	0	0	3	1	0	0	0	1	2	0	0	0	2	2	0	0	0	2
08:30	08:45	27	114	0.79	0.0%	0.0%	0.0%	0	0	0	0	0	4	0	0	0	4	2	0	0	0	2	1	0	0	0	1	3	0	0	0	3
08:45	09:00	30	125	0.87	0.0%	0.0%	0.0%	0	0	0	0	0	3	0	0	0	3	1	0	0	0	1	4	0	0	0	4	8	0	0	0	8
15:00	15:15	32						0	0	0	0	0	6	0	0	0	6	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2
15:15	15:30	40						0	0	0	0	0	5	0	1	0	4	2	0	0	0	2	2	0	0	0	2	7	0	0	0	7
15:30	15:45	30						0	0	0	0	0	3	0	0	0	3	3	0	1	0	2	6	0	0	0	6	1	0	0	0	1
15:45	16:00	38	140	0.88	0.0%	2.6%	0.0%	0	0	0	0	0	5	0	1	0	4	3	0	0	0	3	2	0	0	0	2	2	0	0	0	2
16:00	16:15	49	157	0.8	0.0%	4.1%	0.0%	0	0	0	0	0	3	0	0	0	3	0	0	0	0	0	3	0	0	0	3	10	0	0	0	10
16:15	16:30	34	151	0.77	0.0%	0.0%	0.0%	0	0	0	0	0	6	0	0	0	6	1	0	0	0	1	1	0	0	0	1	3	0	0	0	3
16:30	16:45	45	166	0.85	0.0%	0.0%	0.0%	1	0	0	0	1	5	0	0	0	5	0	0	0	0	0	1	0	0	0	1	2	0	0	0	2
16:45	17:00	44	172	0.88	0.0%	0.0%	0.0%	0	0	0	0	0	4	0	0	0	4	3	0	0	0	3	2	0	0	0	2	11	0	0	0	11
17:00	17:15	43	166	0.92	0.0%	0.0%	0.0%	0	0	0	0	0	6	0	0	0	6	1	0	0	0	1	1	0	0	0	1	5	0	0	0	5
17:15	17:30	26	158	0.88	0.0%	0.0%	0.0%	0	0	0	0	0	4	0	0	0	4	0	0	0	0	0	1	0	0	0	1	2	0	0	0	2
17:30	17:45	25	138	0.78	0.0%	4.0%	0.0%	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	1	0	0	0	1	3	0	0	0	3
17:45	18:00	15	109	0.63	0.0%	0.0%	0.0%	0	0	0	0	0	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2

15 min inter	min survey ntervals 6							7					8					9					10					11					12			
Start	End	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Taxi	Light	Total	Bus	Heavy	Ta xi	Light
06:00	06:15	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15	06:30	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:30	06:45	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45	07:00	2	0	0	0	2	1	0	0	0	1	4	0	0	0	4	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00	07:15	5	0	0	0	5	1	0	0	0	1	3	0	0	0	3	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0
07:15	07:30	5	0	0	0	5	3	0	0	0	3	4	0	0	0	4	0	0	0	0	0	1	0	0	0	1	6	0	0	0	6	0	0	0	0	0
07:30	07:45	1	0	0	0	1	4	0	0	0	4	2	0	0	0	2	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0
07:45	08:00	3	0	0	0	3	2	0	0	0	2	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0
08:00	08:15	2	0	0	0	2	7	0	0	0	7	4	0	0	0	4	2	0	0	0	2	1	0	0	0	1	4	0	0	0	4	0	0	0	0	0
08:15	08:30	8	0	0	0	8	5	0	0	0	5	5	0	0	0	5	0	0	0	0	0	1	0	0	0	1	4	0	0	0	4	1	0	0	0	1
08:30	08:45	4	0	0	0	4	7	0	0	0	7	2	0	0	0	2	0	0	0	0	0	1	0	0	0	1	3	0	0	0	3	0	0	0	0	0
08:45	09:00	7	0	0	0	7	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	2	0	0	0	2	0	0	0	0	0
15:00	15:15	6	0	0	0	6	4	0	0	0	4	5	0	0	0	5	3	0	0	0	3	2	0	0	0	2	3	0	0	0	3	1	0	0	0	1
15:15	15:30	5	0	0	0	5	3	0	0	0	3	8	0	2	0	6	3	0	0	0	3	1	0	0	0	1	4	0	0	0	4	0	0	0	0	0
15:30	15:45	2	0	0	0	2	7	0	1	0	6	2	0	0	0	2	0	0	0	0	0	3	0	0	0	3	3	0	0	0	3	0	0	0	0	0
15:45	16:00	6	0	0	0	6	10	0	0	0	10	3	0	0	0	3	1	0	0	0	1	1	0	0	0	1	5	0	0	0	5	0	0	0	0	0
16:00	16:15	2	0	1	0	1	9	0	0	0	9	8	0	0	0	8	3	0	1	0	2	2	0	0	0	2	8	0	0	0	8	1	0	0	0	1
16:15	16:30	3	0	0	0	3	6	0	0	0	6	5	0	0	0	5	1	0	0	0	1	0	0	0	0	0	6	0	0	0	6	2	0	0	0	2
16:30	16:45	7	0	0	0	7	11	0	0	0	11	7	0	0	0	7	2	0	0	0	2	2	0	0	0	2	7	0	0	0	7	0	0	0	0	0
16:45	17:00	5	0	0	0	5	7	0	0	0	7	5	0	0	0	5	0	0	0	0	0	2	0	0	0	2	4	0	0	0	4	1	0	0	0	1
17:00	17:15	2	0	0	0	2	13	0	0	0	13	7	0	0	0	7	2	0	0	0	2	2	0	0	0	2	4	0	0	0	4	0	0	0	0	0
17:15	17:30	5	0	0	0	5	1	0	0	0	1	7	0	0	0	7	2	0	0	0	2	2	0	0	0	2	2	0	0	0	2	0	0	0	0	0
17:30	17:45	3	0	0	0	3	4	0	0	0	4	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	5	0	1	0	4	0	0	0	0	0
17:45	18:00	3	0	0	0	3	3	0	0	0	3	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Appendix B Detailed SIDRA Outputs

Main Road and Buitekant Street

MOVEMENT SUMMARY

Site: 1 [Main/Buitekant 2021 Base AM]

New Site Site Category: (None) Stop (All-Way)

Move	ment Perfo	mance - V	/ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Main Road											
1	L2	79	3.0	0.748	30.6	LOS D	5.5	39.4	1.00	1.77	4.37	40.3
2	T1	257	3.0	0.748	30.3	LOS D	5.5	39.4	1.00	1.77	4.37	40.1
3	R2	2	3.0	0.005	9.8	LOS A	0.0	0.1	0.85	1.23	1.85	51.0
Approa	ach	338	3.0	0.748	30.2	LOS D	5.5	39.4	1.00	1.77	4.36	40.2
East: E	Buitekant Stre	et										
4	L2	2	3.0	0.077	25.3	LOS D	0.3	1.9	0.98	1.25	2.06	42.6
5	T1	3	3.0	0.077	24.9	LOS C	0.3	1.9	0.98	1.25	2.06	42.5
6	R2	11	3.0	0.077	25.0	LOS D	0.3	1.9	0.98	1.25	2.06	42.5
Approa	ach	16	3.0	0.077	25.0	LOS D	0.3	1.9	0.98	1.25	2.06	42.5
North:	Main Road											
7	L2	4	3.0	0.462	15.9	LOS C	2.1	15.1	0.91	1.43	2.72	48.0
8	T1	227	3.0	0.462	15.6	LOS C	2.1	15.1	0.91	1.43	2.72	47.8
9	R2	72	3.0	0.157	10.8	LOS B	0.5	3.8	0.86	1.27	2.04	50.3
Approa	ach	303	3.0	0.462	14.5	LOS B	2.1	15.1	0.90	1.39	2.56	48.4
West:	Buitekant Str	eet										
10	L2	57	3.0	1.292	293.0	LOS F	11.2	80.6	1.00	1.94	5.10	10.3
11	T1	3	3.0	1.292	292.7	LOS F	11.2	80.6	1.00	1.94	5.10	10.3
12	R2	42	3.0	1.292	292.8	LOS F	11.2	80.6	1.00	1.94	5.10	10.3
Approa	ach	102	3.0	1.292	292.9	LOS F	11.2	80.6	1.00	1.94	5.10	10.3
All Veh	icles	759	3.0	1.292	59.1	LOS F	11.2	80.6	0.96	1.63	3.69	30.4

MOVEMENT SUMMARY

Site: 1 [Main/Buitekant 2021 Base PM]

Moven	nent Perfor	mance - V	/ehicles									
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: I	Main Road											
1	L2	91	3.0	0.937	59.6	LOS F	10.8	77.6	1.00	2.34	6.82	30.5
2	T1	286	3.0	0.937	59.3	LOS F	10.8	77.6	1.00	2.34	6.82	30.4
3	R2	5	3.0	0.014	10.4	LOS B	0.0	0.3	0.88	1.23	1.89	50.6
Approa	ch	382	3.0	0.937	58.7	LOS F	10.8	77.6	1.00	2.32	6.75	30.6
East: Bu	uitekant Stre	et										
4	L2	3	3.0	0.091	30.7	LOS D	0.3	2.3	1.00	1.25	2.09	40.1
5	T1	5	3.0	0.091	30.4	LOS D	0.3	2.3	1.00	1.25	2.09	40.0
6	R2	6	3.0	0.091	30.5	LOS D	0.3	2.3	1.00	1.25	2.09	40.0
Approa	ch	15	3.0	0.091	30.5	LOS D	0.3	2.3	1.00	1.25	2.09	40.0
North: N	/lain Road											
7	L2	15	3.0	0.649	23.6	LOS C	3.9	28.1	0.98	1.60	3.61	43.7
8	T1	283	3.0	0.649	23.3	LOS C	3.9	28.1	0.98	1.60	3.61	43.5
9	R2	94	3.0	0.224	12.1	LOS B	0.8	5.8	0.89	1.29	2.17	49.4
Approa	ch	392	3.0	0.649	20.6	LOS C	3.9	28.1	0.96	1.53	3.27	44.8
West: B	uitekant Stre	et										
10	L2	75	3.0	0.846	78.1	LOS F	6.7	47.9	1.00	1.84	4.64	26.4
11	T1	2	3.0	0.846	77.8	LOS F	6.7	47.9	1.00	1.84	4.64	26.3
12	R2	96	3.0	0.846	77.9	LOS F	6.7	47.9	1.00	1.84	4.64	26.3
Approa	ch	173	3.0	0.846	78.0	LOS F	6.7	47.9	1.00	1.84	4.64	26.3
All Vehi	cles	961	3.0	0.937	46.2	LOS E	10.8	77.6	0.98	1.90	4.88	34.1

Site: 1 [Main/Buitekant 2021 Base + Dev AM - Existing]

New Site Site Category: (None) Stop (All-Way)

Move	ment Perfo	rmance -	Vehicle	es								
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Main Road											
1	L2	102	3.0	0.895	51.9	LOS F	9.2	66.3	1.00	2.17	6.11	32.6
2	T1	257	3.0	0.895	51.6	LOS F	9.2	66.3	1.00	2.17	6.11	32.5
3	R2	2	3.0	0.006	10.3	LOS B	0.0	0.1	0.88	1.23	1.88	50.7
Approa	ich	361	3.0	0.895	51.4	LOS F	9.2	66.3	1.00	2.17	6.08	32.6
East: E	Buitekant Str	eet										
4	L2	2	3.0	0.056	19.6	LOS C	0.2	1.3	0.94	1.24	1.99	45.6
5	T1	3	3.0	0.056	19.3	LOS C	0.2	1.3	0.94	1.24	1.99	45.4
6	R2	11	3.0	0.056	19.4	LOS C	0.2	1.3	0.94	1.24	1.99	45.4
Approa	ich	16	3.0	0.056	19.4	LOS C	0.2	1.3	0.94	1.24	1.99	45.4
North:	Main Road											
7	L2	4	3.0	0.495	17.5	LOS C	2.4	17.0	0.93	1.45	2.85	47.1
8	T1	227	3.0	0.495	17.2	LOS C	2.4	17.0	0.93	1.45	2.85	46.8
9	R2	93	3.0	0.220	12.0	LOS B	0.8	5.7	0.89	1.29	2.16	49.5
Approa	ich	324	3.0	0.495	15.7	LOS C	2.4	17.0	0.92	1.40	2.66	47.6
West:	Buitekant St	reet										
10	L2	135	3.0	1.226	189.1	LOS F	17.0	122.1	1.00	2.65	8.19	14.6
11	T1	3	3.0	1.226	188.8	LOS F	17.0	122.1	1.00	2.65	8.19	14.6
12	R2	99	3.0	1.226	188.9	LOS F	17.0	122.1	1.00	2.65	8.19	14.6
Approa	ach	237	3.0	1.226	189.0	LOS F	17.0	122.1	1.00	2.65	8.19	14.6
All Veh	icles	938	3.0	1.226	73.3	LOS F	17.0	122.1	0.97	2.01	5.36	27.2

MOVEMENT SUMMARY

Site: 1 [Main/Buitekant 2021 Base + Dev PM - Existing]

Move	ment Perfo	rmance -	Vehicle	es								
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Main Road											
1	L2	152	3.0	1.159	125.9	LOS F	22.3	160.0	1.00	3.39	11.40	19.6
2	T1	286	3.0	1.159	125.6	LOS F	22.3	160.0	1.00	3.39	11.40	19.6
3	R2	5	3.0	0.015	10.7	LOS B	0.0	0.4	0.89	1.23	1.91	50.4
Approa	ich	443	3.0	1.159	124.4	LOS F	22.3	160.0	1.00	3.36	11.28	19.7
East: E	Buitekant Str	eet										
4	L2	3	3.0	0.087	29.4	LOS D	0.3	2.2	1.00	1.25	2.09	40.7
5	T1	5	3.0	0.087	29.1	LOS D	0.3	2.2	1.00	1.25	2.09	40.6
6	R2	6	3.0	0.087	29.2	LOS D	0.3	2.2	1.00	1.25	2.09	40.6
Approa	ich	15	3.0	0.087	29.2	LOS D	0.3	2.2	1.00	1.25	2.09	40.6
North:	Main Road											
7	L2	15	3.0	0.643	23.2	LOS C	3.8	27.6	0.98	1.60	3.57	43.9
8	T1	283	3.0	0.643	22.9	LOS C	3.8	27.6	0.98	1.60	3.57	43.7
9	R2	158	3.0	0.375	14.7	LOS B	1.6	11.2	0.92	1.36	2.49	47.8
Approa	ich	456	3.0	0.643	20.0	LOS C	3.8	27.6	0.96	1.52	3.20	45.0
West: I	Buitekant Sti	reet										
10	L2	98	3.0	1.031	116.9	LOS F	11.5	82.7	1.00	2.28	6.58	20.6
11	T1	2	3.0	1.031	116.5	LOS F	11.5	82.7	1.00	2.28	6.58	20.6
12	R2	126	3.0	1.031	116.6	LOS F	11.5	82.7	1.00	2.28	6.58	20.6
Approa	ich	226	3.0	1.031	116.7	LOS F	11.5	82.7	1.00	2.28	6.58	20.6
All Veh	icles	1140	3.0	1.159	79.9	LOS F	22.3	160.0	0.98	2.38	7.00	25.9

Site: 1 [Main/Buitekant 2021 Base + Dev AM - Proposed]

New Site Site Category: (None) Roundabout

Move	ment Perfo	rmance -	Vehicle	es								
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Main Road											
1	L2	102	3.0	0.097	4.2	LOS A	0.5	3.4	0.28	0.45	0.28	55.4
2	T1	257	3.0	0.183	4.3	LOS A	1.0	7.2	0.27	0.41	0.27	56.2
3	R2	2	3.0	0.183	9.5	LOS A	1.0	7.2	0.27	0.41	0.27	56.6
Approa	ich	361	3.0	0.183	4.3	LOS A	1.0	7.2	0.27	0.42	0.27	56.0
East: E	Buitekant Str	eet										
4	L2	2	3.0	0.019	5.9	LOS A	0.1	0.6	0.48	0.63	0.48	51.5
5	T1	3	3.0	0.019	5.9	LOS A	0.1	0.6	0.48	0.63	0.48	53.2
6	R2	11	3.0	0.019	11.0	LOS B	0.1	0.6	0.48	0.63	0.48	53.0
Approa	ach	16	3.0	0.019	9.3	LOS A	0.1	0.6	0.48	0.63	0.48	52.8
North:	Main Road											
7	L2	4	3.0	0.082	4.3	LOS A	0.4	3.0	0.30	0.41	0.30	54.8
8	T1	227	3.0	0.171	4.4	LOS A	1.0	7.1	0.29	0.47	0.29	55.3
9	R2	93	3.0	0.171	9.5	LOS A	1.0	7.1	0.28	0.50	0.28	55.2
Approa	ich	324	3.0	0.171	5.8	LOS A	1.0	7.1	0.29	0.48	0.29	55.2
West: I	Buitekant Sti	reet										
10	L2	135	3.0	0.253	5.7	LOS A	1.4	10.1	0.50	0.65	0.50	52.5
11	T1	3	3.0	0.253	5.7	LOS A	1.4	10.1	0.50	0.65	0.50	54.2
12	R2	99	3.0	0.253	10.8	LOS B	1.4	10.1	0.50	0.65	0.50	54.1
Approa	ach	237	3.0	0.253	7.9	LOS A	1.4	10.1	0.50	0.65	0.50	53.2
All Veh	icles	938	3.0	0.253	5.8	LOS A	1.4	10.1	0.34	0.50	0.34	54.9

MOVEMENT SUMMARY

Site: 1 [Main/Buitekant 2021 Base + Dev PM - Proposed]

New Site Site Category: (None) Roundabout

Move	ment Perfo	rmance -	Vehicle	es								
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Main Road											
1	L2	152	3.0	0.142	4.6	LOS A	0.7	5.3	0.37	0.49	0.37	55.0
2	T1	286	3.0	0.220	4.6	LOS A	1.3	9.1	0.36	0.45	0.36	55.7
3	R2	5	3.0	0.220	9.9	LOS A	1.3	9.1	0.36	0.45	0.36	56.0
Approa	ach	443	3.0	0.220	4.6	LOS A	1.3	9.1	0.36	0.47	0.36	55.5
East: E	Buitekant Str	eet										
4	L2	3	3.0	0.019	6.6	LOS A	0.1	0.6	0.55	0.64	0.55	51.8
5	T1	5	3.0	0.019	6.6	LOS A	0.1	0.6	0.55	0.64	0.55	53.5
6	R2	6	3.0	0.019	11.7	LOS B	0.1	0.6	0.55	0.64	0.55	53.3
Approa	ach	15	3.0	0.019	8.8	LOS A	0.1	0.6	0.55	0.64	0.55	53.0
North:	Main Road											
7	L2	15	3.0	0.118	4.5	LOS A	0.6	4.5	0.35	0.44	0.35	54.6
8	T1	283	3.0	0.247	4.6	LOS A	1.5	11.0	0.34	0.50	0.34	54.9
9	R2	158	3.0	0.247	9.7	LOS A	1.5	11.0	0.34	0.54	0.34	54.6
Approa	ach	456	3.0	0.247	6.3	LOS A	1.5	11.0	0.34	0.51	0.34	54.8
West:	Buitekant St	reet										
10	L2	98	3.0	0.250	5.9	LOS A	1.4	10.1	0.53	0.68	0.53	51.9
11	T1	2	3.0	0.250	5.9	LOS A	1.4	10.1	0.53	0.68	0.53	53.6
12	R2	126	3.0	0.250	11.0	LOS B	1.4	10.1	0.53	0.68	0.53	53.5
Approa	ach	226	3.0	0.250	8.8	LOS A	1.4	10.1	0.53	0.68	0.53	52.8
All Veh	icles	1140	3.0	0.250	6.2	LOS A	1.5	11.0	0.39	0.53	0.39	54.6

𝔅 Site: 1 [Main/Buitekant 2026 AM - Proposed]

New Site Site Category: (None) Roundabout

Move	ment Perfo	rmance - V	/ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Main Road											
1	L2	102	3.0	0.098	4.3	LOS A	0.5	3.5	0.29	0.45	0.29	55.4
2	T1	269	3.0	0.192	4.3	LOS A	1.1	7.6	0.27	0.41	0.27	56.2
3	R2	2	3.0	0.192	9.5	LOS A	1.1	7.6	0.27	0.41	0.27	56.6
Appro	ach	374	3.0	0.192	4.3	LOS A	1.1	7.6	0.28	0.42	0.28	56.0
East: I	Buitekant Stre	et										
4	L2	2	3.0	0.019	5.9	LOS A	0.1	0.6	0.49	0.63	0.49	51.5
5	T1	3	3.0	0.019	5.9	LOS A	0.1	0.6	0.49	0.63	0.49	53.1
6	R2	11	3.0	0.019	11.0	LOS B	0.1	0.6	0.49	0.63	0.49	53.0
Appro	ach	16	3.0	0.019	9.3	LOS A	0.1	0.6	0.49	0.63	0.49	52.8
North:	Main Road											
7	L2	4	3.0	0.085	4.3	LOS A	0.4	3.2	0.30	0.41	0.30	54.8
8	T1	239	3.0	0.178	4.4	LOS A	1.0	7.5	0.29	0.47	0.29	55.3
9	R2	93	3.0	0.178	9.5	LOS A	1.0	7.5	0.28	0.50	0.28	55.2
Appro	ach	336	3.0	0.178	5.8	LOS A	1.0	7.5	0.29	0.48	0.29	55.3
West:	Buitekant Str	eet										
10	L2	135	3.0	0.257	5.8	LOS A	1.4	10.3	0.51	0.66	0.51	52.4
11	T1	3	3.0	0.257	5.8	LOS A	1.4	10.3	0.51	0.66	0.51	54.2
12	R2	99	3.0	0.257	10.9	LOS B	1.4	10.3	0.51	0.66	0.51	54.0
Appro	ach	237	3.0	0.257	8.0	LOS A	1.4	10.3	0.51	0.66	0.51	53.1
All Vel	hicles	962	3.0	0.257	5.8	LOS A	1.4	10.3	0.34	0.50	0.34	54.9

MOVEMENT SUMMARY

Site: 1 [Main/Buitekant 2026 PM - Proposed]

New Site Site Category: (None) Roundabout

Moven	ent Perfor	mance - V	/ehicles									
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: I	Vain Road											
1	L2	152	3.0	0.144	4.6	LOS A	0.8	5.4	0.37	0.49	0.37	55.0
2	T1	301	3.0	0.231	4.6	LOS A	1.3	9.7	0.37	0.45	0.37	55.7
3	R2	5	3.0	0.231	9.9	LOS A	1.3	9.7	0.37	0.45	0.37	56.0
Approa	ch	458	3.0	0.231	4.7	LOS A	1.3	9.7	0.37	0.47	0.37	55.5
East: B	uitekant Stree	et										
4	L2	3	3.0	0.020	6.7	LOS A	0.1	0.6	0.56	0.64	0.56	51.7
5	T1	5	3.0	0.020	6.6	LOS A	0.1	0.6	0.56	0.64	0.56	53.4
6	R2	6	3.0	0.020	11.7	LOS B	0.1	0.6	0.56	0.64	0.56	53.3
Approa	ch	15	3.0	0.020	8.8	LOS A	0.1	0.6	0.56	0.64	0.56	53.0
North: N	Aain Road											
7	L2	15	3.0	0.122	4.5	LOS A	0.7	4.7	0.35	0.44	0.35	54.6
8	T1	298	3.0	0.254	4.6	LOS A	1.6	11.5	0.35	0.50	0.35	54.9
9	R2	158	3.0	0.254	9.7	LOS A	1.6	11.5	0.35	0.54	0.35	54.7
Approa	ch	471	3.0	0.254	6.3	LOS A	1.6	11.5	0.35	0.51	0.35	54.8
West: B	uitekant Stre	et										
10	L2	98	3.0	0.253	6.0	LOS A	1.4	10.3	0.54	0.69	0.54	51.8
11	T1	2	3.0	0.253	6.0	LOS A	1.4	10.3	0.54	0.69	0.54	53.5
12	R2	126	3.0	0.253	11.1	LOS B	1.4	10.3	0.54	0.69	0.54	53.4
Approa	ch	226	3.0	0.253	8.9	LOS A	1.4	10.3	0.54	0.69	0.54	52.7
All Vehi	cles	1169	3.0	0.254	6.2	LOS A	1.6	11.5	0.39	0.53	0.39	54.6

Buitekant Street and Excelsior Street

MOVEMENT SUMMARY

Site: 2 [Buitekant/Excelsior 2021 Base AM]

New Site Site Category: (None) Stop (Two-Way)

Moverr	ent Perfor	nance - V	<i>lehicles</i>									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	uitekant Stree	t										
5	T1	60	3.0	0.089	0.4	LOS A	0.4	3.0	0.24	0.35	0.24	56.1
6	R2	94	0.0	0.089	5.9	LOS A	0.4	3.0	0.24	0.35	0.24	54.1
Approad	h	154	1.2	0.089	3.7	NA	0.4	3.0	0.24	0.35	0.24	54.8
North: Excelsior Stre		et										
7	L2	1	0.0	0.002	8.4	LOS A	0.0	0.0	0.22	0.86	0.22	51.7
9	R2	1	0.0	0.002	8.6	LOS A	0.0	0.0	0.22	0.86	0.22	51.2
Approad	h	2	0.0	0.002	8.5	LOS A	0.0	0.0	0.22	0.86	0.22	51.5
West: B	uitekant Stre	et										
10	L2	29	0.0	0.069	5.5	LOS A	0.0	0.0	0.00	0.13	0.00	57.2
11	T1	102	3.0	0.069	0.0	LOS A	0.0	0.0	0.00	0.13	0.00	58.8
Approad	h	132	2.3	0.069	1.2	NA	0.0	0.0	0.00	0.13	0.00	58.4
All Vehic	cles	287	1.7	0.089	2.6	NA	0.4	3.0	0.13	0.26	0.13	56.4

MOVEMENT SUMMARY

Site: 2 [Buitekant/Excelsior 2021 Base PM]

Moven	nent Perform	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: B	uitekant Stree	ŧ										
5	T1	85	3.0	0.106	0.5	LOS A	0.5	3.6	0.29	0.31	0.29	56.3
6	R2	94	0.0	0.106	6.1	LOS A	0.5	3.6	0.29	0.31	0.29	54.3
Approa	ch	179	1.4	0.106	3.5	NA	0.5	3.6	0.29	0.31	0.29	55.2
North: Excelsior Stre		et										
7	L2	1	0.0	0.002	8.6	LOS A	0.0	0.1	0.30	0.84	0.30	51.5
9	R2	1	0.0	0.002	9.1	LOS A	0.0	0.1	0.30	0.84	0.30	51.1
Approa	ch	2	0.0	0.002	8.9	LOS A	0.0	0.1	0.30	0.84	0.30	51.3
West: B	uitekant Stree	et										
10	L2	29	0.0	0.106	5.6	LOS A	0.0	0.0	0.00	0.09	0.00	57.6
11	T1	173	3.0	0.106	0.0	LOS A	0.0	0.0	0.00	0.09	0.00	59.2
Approa	ch	202	2.6	0.106	0.8	NA	0.0	0.0	0.00	0.09	0.00	58.9
All Vehi	cles	383	2.0	0.106	2.1	NA	0.5	3.6	0.14	0.20	0.14	57.1

Site: 2 [Buitekant/Excelsior 2021 Base + Dev AM - Existing]

New Site Site Category: (None) Stop (Two-Way)

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: E	Buitekant S	street										
5	T1	104	3.0	0.119	0.7	LOS A	0.6	4.0	0.33	0.29	0.33	56.3
6	R2	94	0.0	0.119	6.4	LOS A	0.6	4.0	0.33	0.29	0.33	54.3
Approa	ach	198	1.6	0.119	3.4	NA	0.6	4.0	0.33	0.29	0.33	55.4
North: Excelsior		Street										
7	L2	1	0.0	0.002	8.9	LOS A	0.0	0.1	0.36	0.83	0.36	51.3
9	R2	1	0.0	0.002	9.6	LOS A	0.0	0.1	0.36	0.83	0.36	50.9
Approa	ach	2	0.0	0.002	9.3	LOS A	0.0	0.1	0.36	0.83	0.36	51.1
West:	Buitekant S	Street										
10	L2	29	0.0	0.140	5.6	LOS A	0.0	0.0	0.00	0.07	0.00	57.8
11	T1	237	3.0	0.140	0.0	LOS A	0.0	0.0	0.00	0.07	0.00	59.4
Approa	ach	266	2.7	0.140	0.6	NA	0.0	0.0	0.00	0.07	0.00	59.2
All Veh	nicles	466	2.2	0.140	1.8	NA	0.6	4.0	0.14	0.17	0.14	57.5

MOVEMENT SUMMARY

Site: 2 [Buitekant/Excelsior 2021 Base + Dev PM - Existing]

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: E	Buitekant S	Street										
5	T1	211	3.0	0.179	0.5	LOS A	0.7	5.3	0.27	0.21	0.27	57.2
6	R2	104	0.0	0.179	6.4	LOS A	0.7	5.3	0.27	0.21	0.27	55.2
Approa	ach	315	2.0	0.179	2.5	NA	0.7	5.3	0.27	0.21	0.27	56.5
North:	Excelsior	Street										
7	L2	1	0.0	0.003	8.8	LOS A	0.0	0.1	0.37	0.83	0.37	51.1
9	R2	1	0.0	0.003	10.4	LOS B	0.0	0.1	0.37	0.83	0.37	50.6
Approa	ach	2	0.0	0.003	9.6	LOS A	0.0	0.1	0.37	0.83	0.37	50.9
West:	Buitekant	Street										
10	L2	26	0.0	0.132	5.6	LOS A	0.0	0.0	0.00	0.06	0.00	57.8
11	T1	226	3.0	0.132	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	59.4
Approa	ach	253	2.7	0.132	0.6	NA	0.0	0.0	0.00	0.06	0.00	59.2
All Veh	icles	569	2.3	0.179	1.7	NA	0.7	5.3	0.15	0.15	0.15	57.7

Site: 2 [Buitekant/Excelsior 2026 AM - Existing]

New Site Site Category: (None) Stop (Two-Way)

Moven	Movement Performance - Vehicles Mov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average														
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h			
East: B	uitekant Stre	et													
5	T1	104	3.0	0.119	0.7	LOS A	0.6	4.0	0.33	0.29	0.33	56.3			
6	R2	94	0.0	0.119	6.4	LOS A	0.6	4.0	0.33	0.29	0.33	54.3			
Approach		198	1.6	0.119	3.4	NA	0.6	4.0	0.33	0.29	0.33	55.4			
North: Excelsior Stre		et													
7	L2	1	0.0	0.002	8.9	LOS A	0.0	0.1	0.36	0.83	0.36	51.3			
9	R2	1	0.0	0.002	9.6	LOS A	0.0	0.1	0.36	0.83	0.36	50.9			
Approa	ch	2	0.0	0.002	9.3	LOS A	0.0	0.1	0.36	0.83	0.36	51.1			
West: E	luitekant Stre	et													
10	L2	29	0.0	0.140	5.6	LOS A	0.0	0.0	0.00	0.07	0.00	57.8			
11	T1	237	3.0	0.140	0.0	LOS A	0.0	0.0	0.00	0.07	0.00	59.4			
Approa	ch	266	2.7	0.140	0.6	NA	0.0	0.0	0.00	0.07	0.00	59.2			
All Vehi	cles	466	2.2	0.140	1.8	NA	0.6	4.0	0.14	0.17	0.14	57.5			

MOVEMENT SUMMARY

Site: 2 [Buitekant/Excelsior 2026 PM - Existing]

Movem	ent Perform	nance - Ve	ehicles									
Mov ID	Tum	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	itekant Street	t										
5	T1	211	3.0	0.179	0.5	LOS A	0.7	5.3	0.27	0.21	0.27	57.2
6	R2	104	0.0	0.179	6.4	LOS A	0.7	5.3	0.27	0.21	0.27	55.2
Approac	h	315	2.0	0.179	2.5	NA	0.7	5.3	0.27	0.21	0.27	56.5
North: Excelsior Stre		t										
7	L2	1	0.0	0.003	8.8	LOS A	0.0	0.1	0.37	0.83	0.37	51.1
9	R2	1	0.0	0.003	10.4	LOS B	0.0	0.1	0.37	0.83	0.37	50.6
Approac	h	2	0.0	0.003	9.6	LOS A	0.0	0.1	0.37	0.83	0.37	50.9
West: B	uitekant Stree	t										
10	L2	26	0.0	0.132	5.6	LOS A	0.0	0.0	0.00	0.06	0.00	57.8
11	T1	226	3.0	0.132	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	59.4
Approac	h	253	2.7	0.132	0.6	NA	0.0	0.0	0.00	0.06	0.00	59.2
All Vehic	les	569	2.3	0.179	1.7	NA	0.7	5.3	0.15	0.15	0.15	57.7

Buitekant Street and Fynbos Street

MOVEMENT SUMMARY

🕮 Site: 3 [Buitekant/Fynbos 2021 Base AM]

New Site Site Category: (None) Stop (All-Way)

Movem	ent Perforn	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: F	ynbos Street											
1	L2	12	3.0	0.132	12.8	LOS B	0.4	3.0	0.76	1.27	1.91	49.6
3	R2	71	3.0	0.132	12.3	LOS B	0.4	3.0	0.76	1.27	1.91	49.2
Approac	Approach East: Buitekant Stree		3.0	0.132	12.3	LOS B	0.4	3.0	0.76	1.27	1.91	49.2
East: Buitekant Stree		t										
4	L2	24	3.0	0.167	18.2	LOS C	0.6	4.2	0.91	1.28	2.10	46.5
5	T1	36	3.0	0.167	17.8	LOS C	0.6	4.2	0.91	1.28	2.10	46.3
Approac	h	60	3.0	0.167	18.0	LOS C	0.6	4.2	0.91	1.28	2.10	46.4
West: B	uitekant Stree	et										
11	T1	61	3.0	0.142	14.5	LOS B	0.5	3.4	0.84	1.28	2.00	48.3
12	R2	7	3.0	0.142	14.3	LOS B	0.5	3.4	0.84	1.28	2.00	48.1
Approac	h	68	3.0	0.142	14.5	LOS B	0.5	3.4	0.84	1.28	2.00	48.3
All Vehic	les	211	3.0	0.167	14.6	LOS B	0.6	4.2	0.83	1.27	1.99	48.1

MOVEMENT SUMMARY

Site: 3 [Buitekant/Fynbos 2021 Base PM]

Moverr	ent Perform	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: F	Fynbos Street	t										
1	L2	44	3.0	0.237	12.7	LOS B	0.8	5.8	0.73	1.30	2.04	49.6
3	R2	125	3.0	0.237	12.2	LOS B	0.8	5.8	0.73	1.30	2.04	49.2
Approad	ch	169	3.0	0.237	12.3	LOS B	0.8	5.8	0.73	1.30	2.04	49.3
East: Bu	uitekant Stree	t										
4	L2	32	3.0	0.256	20.7	LOS C	1.0	7.0	0.94	1.31	2.27	45.1
5	T1	54	3.0	0.256	20.3	LOS C	1.0	7.0	0.94	1.31	2.27	44.9
Approad	ch	85	3.0	0.256	20.5	LOS C	1.0	7.0	0.94	1.31	2.27	45.0
West: B	uitekant Stree	et										
11	T1	74	3.0	0.193	15.6	LOS C	0.7	4.8	0.86	1.29	2.10	47.7
12	R2	14	3.0	0.193	15.4	LOS C	0.7	4.8	0.86	1.29	2.10	47.5
Approad	ch	87	3.0	0.193	15.6	LOS C	0.7	4.8	0.86	1.29	2.10	47.6
All Vehic	cles	342	3.0	0.256	15.2	LOS C	1.0	7.0	0.82	1.30	2.11	47.7

Site: 3 [Buitekant/Fynbos 2021 Base + Dev AM - Existing]

New Site Site Category: (None) Stop (All-Way)

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Fynbos S	treet										
1	L2	12	3.0	0.195	16.8	LOS C	0.7	5.0	0.88	1.28	2.12	47.3
3	R2	71	3.0	0.195	16.2	LOS C	0.7	5.0	0.88	1.28	2.12	46.8
Approa	ach	82	3.0	0.195	16.3	LOS C	0.7	5.0	0.88	1.28	2.12	46.9
East: Buitekant S		Street										
4	L2	24	3.0	0.209	15.3	LOS C	0.7	5.3	0.84	1.30	2.10	48.3
5	T1	80	3.0	0.209	14.9	LOS B	0.7	5.3	0.84	1.30	2.10	48.0
Approa	ach	104	3.0	0.209	15.0	LOS B	0.7	5.3	0.84	1.30	2.10	48.1
West:	Buitekant S	Street										
11	T1	196	3.0	0.322	14.2	LOS B	1.2	8.8	0.81	1.36	2.27	48.5
12	R2	7	3.0	0.322	14.0	LOS B	1.2	8.8	0.81	1.36	2.27	48.3
Approa	ach	203	3.0	0.322	14.2	LOS B	1.2	8.8	0.81	1.36	2.27	48.5
All Veh	nicles	389	3.0	0.322	14.9	LOS B	1.2	8.8	0.83	1.33	2.19	48.0

MOVEMENT SUMMARY

Site: 3 [Buitekant/Fynbos 2021 Base + Dev PM - Existing]

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Fynbos St	treet										
1	L2	44	3.0	0.318	16.0	LOS C	1.2	8.8	0.86	1.33	2.31	47.7
3	R2	125	3.0	0.318	15.5	LOS C	1.2	8.8	0.86	1.33	2.31	47.2
Approa	ach	169	3.0	0.318	15.6	LOS C	1.2	8.8	0.86	1.33	2.31	47.3
East: E	Buitekant S	street										
4	L2	32	3.0	0.410	17.9	LOS C	1.8	12.6	0.89	1.39	2.56	46.7
5	T1	179	3.0	0.410	17.6	LOS C	1.8	12.6	0.89	1.39	2.56	46.5
Approa	ach	211	3.0	0.410	17.7	LOS C	1.8	12.6	0.89	1.39	2.56	46.5
West: I	Buitekant S	Street										
11	T1	127	3.0	0.265	15.0	LOS C	1.0	7.0	0.84	1.33	2.19	48.0
12	R2	14	3.0	0.265	14.8	LOS B	1.0	7.0	0.84	1.33	2.19	47.8
Approa	ach	141	3.0	0.265	15.0	LOS B	1.0	7.0	0.84	1.33	2.19	48.0
All Veh	icles	521	3.0	0.410	16.3	LOS C	1.8	12.6	0.87	1.36	2.38	47.2

Site: 3 [Buitekant/Fynbos 2026 AM - Existing]

New Site Site Category: (None) Stop (All-Way)

Moverr	nent Perforr	nance - V	/ehicles									
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: F	Fynbos Street	t										
1	L2	12	3.0	0.195	16.8	LOS C	0.7	5.0	0.88	1.28	2.12	47.3
3	R2	71	3.0	0.195	16.2	LOS C	0.7	5.0	0.88	1.28	2.12	46.8
Approa	Approach East: Buitekant Stree		3.0	0.195	16.3	LOS C	0.7	5.0	0.88	1.28	2.12	46.9
East: Buitekant Stree		et										
4	L2	24	3.0	0.209	15.3	LOS C	0.7	5.3	0.84	1.30	2.10	48.3
5	T1	80	3.0	0.209	14.9	LOS B	0.7	5.3	0.84	1.30	2.10	48.0
Approa	ch	104	3.0	0.209	15.0	LOS B	0.7	5.3	0.84	1.30	2.10	48.1
West: B	uitekant Stre	et										
11	T1	196	3.0	0.322	14.2	LOS B	1.2	8.8	0.81	1.36	2.27	48.5
12	R2	7	3.0	0.322	14.0	LOS B	1.2	8.8	0.81	1.36	2.27	48.3
Approa	ch	203	3.0	0.322	14.2	LOS B	1.2	8.8	0.81	1.36	2.27	48.5
All Vehi	cles	389	3.0	0.322	14.9	LOS B	1.2	8.8	0.83	1.33	2.19	48.0

MOVEMENT SUMMARY

Site: 3 [Buitekant/Fynbos 2026 PM - Existing]

Movem	ent Perform	nance - Ve	hicles									
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: F	ynbos Street											
1	L2	44	3.0	0.318	16.0	LOS C	1.2	8.8	0.86	1.33	2.31	47.7
3	R2	125	3.0	0.318	15.5	LOS C	1.2	8.8	0.86	1.33	2.31	47.2
Approach East: Buitekant Stree		169	3.0	0.318	15.6	LOS C	1.2	8.8	0.86	1.33	2.31	47.3
East: Bu	itekant Street											
4	L2	32	3.0	0.410	17.9	LOS C	1.8	12.6	0.89	1.39	2.56	46.7
5	T1	179	3.0	0.410	17.6	LOS C	1.8	12.6	0.89	1.39	2.56	46.5
Approac	h	211	3.0	0.410	17.7	LOS C	1.8	12.6	0.89	1.39	2.56	46.5
West: Bu	uitekant Stree	t										
11	T1	127	3.0	0.265	15.0	LOS C	1.0	7.0	0.84	1.33	2.19	48.0
12	R2	14	3.0	0.265	14.8	LOS B	1.0	7.0	0.84	1.33	2.19	47.8
Approac	h	141	3.0	0.265	15.0	LOS B	1.0	7.0	0.84	1.33	2.19	48.0
All Vehic	les	521	3.0	0.410	16.3	LOS C	1.8	12.6	0.87	1.36	2.38	47.2

Buitekant Street and Pastorie Street

MOVEMENT SUMMARY

Site: 4 [Buitekant/Pastorie 2021 Base AM]

New Site Site Category: (None) Stop (Two-Way)

Movem	ent Perform	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	itekant Stree	t										
5	T1	38	3.0	0.024	0.0	LOS A	0.1	0.4	0.05	0.12	0.05	58.7
6	R2	9	0.0	0.024	5.6	LOS A	0.1	0.4	0.05	0.12	0.05	56.5
Approac	Approach North: Pastorie Stree		2.4	0.024	1.1	NA	0.1	0.4	0.05	0.12	0.05	58.3
North: P	astorie Stree	t										
7	L2	26	0.0	0.020	8.2	LOS A	0.1	0.5	0.12	0.91	0.12	51.8
9	R2	1	0.0	0.020	7.8	LOS A	0.1	0.5	0.12	0.91	0.12	51.3
Approac	h	27	0.0	0.020	8.1	LOS A	0.1	0.5	0.12	0.91	0.12	51.7
West: B	uitekant Stree	et										
10	L2	1	0.0	0.023	5.5	LOS A	0.0	0.0	0.00	0.01	0.00	58.2
11	T1	42	3.0	0.023	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
Approac	h	43	2.9	0.023	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.8
All Vehic	les	118	2.0	0.024	2.4	NA	0.1	0.5	0.05	0.27	0.05	57.1

MOVEMENT SUMMARY

🕮 Site: 4 [Buitekant/Pastorie 2021 Base PM]

Movem	ent Perforn	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	itekant Stree	t										
5	T1	69	3.0	0.051	0.1	LOS A	0.2	1.1	0.08	0.17	0.08	58.1
6	R2	28	0.0	0.051	5.6	LOS A	0.2	1.1	0.08	0.17	0.08	56.0
Approac	h	98	2.1	0.051	1.7	NA	0.2	1.1	0.08	0.17	0.08	57.5
North: P	astorie Stree	t										
7	L2	43	0.0	0.032	8.2	LOS A	0.1	0.9	0.12	0.91	0.12	51.8
9	R2	1	0.0	0.032	8.1	LOS A	0.1	0.9	0.12	0.91	0.12	51.3
Approac	h	44	0.0	0.032	8.2	LOS A	0.1	0.9	0.12	0.91	0.12	51.7
West: B	uitekant Stree	et										
10	L2	1	0.0	0.024	5.5	LOS A	0.0	0.0	0.00	0.01	0.00	58.2
11	T1	44	3.0	0.024	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
Approac	h	45	2.9	0.024	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.8
All Vehic	les	187	1.8	0.051	2.8	NA	0.2	1.1	0.07	0.31	0.07	56.5

Site: 4 [Buitekant/Pastorie 2021 Base + Dev AM - Existing]

New Site Site Category: (None) Stop (Two-Way)

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: E	Buitekant S	Street										
5	T1	82	3.0	0.047	0.1	LOS A	0.1	0.5	0.07	0.06	0.07	59.1
6	R2	9	0.0	0.047	6.0	LOS A	0.1	0.5	0.07	0.06	0.07	56.9
Approa	ach	92	2.7	0.047	0.7	NA	0.1	0.5	0.07	0.06	0.07	58.9
North: Pastorie Str		Street										
7	L2	26	0.0	0.023	8.7	LOS A	0.1	0.6	0.28	0.86	0.28	51.7
9	R2	1	0.0	0.023	8.7	LOS A	0.1	0.6	0.28	0.86	0.28	51.2
Approa	ach	27	0.0	0.023	8.7	LOS A	0.1	0.6	0.28	0.86	0.28	51.7
West:	Buitekant	Street										
10	L2	1	0.0	0.093	5.5	LOS A	0.0	0.0	0.00	0.00	0.00	58.3
11	T1	177	3.0	0.093	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Approa	ach	178	3.0	0.093	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
All Veh	nicles	297	2.6	0.093	1.0	NA	0.1	0.6	0.05	0.10	0.05	58.7

MOVEMENT SUMMARY

Site: 4 [Buitekant/Pastorie 2021 Base + Dev PM - Existing]

Move	ment Per	formance	- Vehic	lovement Performance - Vehicles lov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average														
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h						
East: E	Buitekant S	itreet																
5	T1	195	3.0	0.114	0.1	LOS A	0.2	1.4	0.06	0.08	0.06	59.0						
6	R2	28	0.0	0.114	5.8	LOS A	0.2	1.4	0.06	0.08	0.06	56.8						
Approa	ich	223	2.6	0.114	0.8	NA	0.2	1.4	0.06	0.08	0.06	58.8						
North: Pastorie S		treet																
7	L2	43	0.0	0.034	8.4	LOS A	0.1	0.9	0.20	0.88	0.20	51.8						
9	R2	1	0.0	0.034	9.0	LOS A	0.1	0.9	0.20	0.88	0.20	51.3						
Approa	ich	44	0.0	0.034	8.4	LOS A	0.1	0.9	0.20	0.88	0.20	51.7						
West: I	Buitekant S	Street																
10	L2	1	0.0	0.052	5.5	LOS A	0.0	0.0	0.00	0.01	0.00	58.3						
11	T1	98	3.0	0.052	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9						
Approa	ich	99	3.0	0.052	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.9						
All Veh	icles	366	2.4	0.114	1.5	NA	0.2	1.4	0.06	0.16	0.06	58.1						

Site: 4 [Buitekant/Pastorie 2026 AM - Existing]

New Site Site Category: (None) Stop (Two-Way)

Moven	nent Perforr	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: B	uitekant Stree	ŧ										
5	T1	82	3.0	0.047	0.1	LOS A	0.1	0.5	0.07	0.06	0.07	59.1
6	R2	9	0.0	0.047	6.0	LOS A	0.1	0.5	0.07	0.06	0.07	56.9
Approa	ch	92	2.7	0.047	0.7	NA	0.1	0.5	0.07	0.06	0.07	58.9
North: Pastorie Stree		t										
7	L2	26	0.0	0.023	8.7	LOS A	0.1	0.6	0.28	0.86	0.28	51.7
9	R2	1	0.0	0.023	8.7	LOS A	0.1	0.6	0.28	0.86	0.28	51.2
Approa	ch	27	0.0	0.023	8.7	LOS A	0.1	0.6	0.28	0.86	0.28	51.7
West: B	uitekant Stre	et										
10	L2	1	0.0	0.093	5.5	LOS A	0.0	0.0	0.00	0.00	0.00	58.3
11	T1	177	3.0	0.093	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Approa	ch	178	3.0	0.093	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
All Vehi	cles	297	2.6	0.093	1.0	NA	0.1	0.6	0.05	0.10	0.05	58.7

MOVEMENT SUMMARY

Site: 4 [Buitekant/Pastorie 2026 PM - Existing]

Movem	ent Perforn	nance - V	ehicles									
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	itekant Stree	t										
5	T1	195	3.0	0.114	0.1	LOS A	0.2	1.4	0.06	0.08	0.06	59.0
6	R2	28	0.0	0.114	5.8	LOS A	0.2	1.4	0.06	0.08	0.06	56.8
Approach		223	2.6	0.114	0.8	NA	0.2	1.4	0.06	0.08	0.06	58.8
North: Pastorie Stree		t										
7	L2	43	0.0	0.034	8.4	LOS A	0.1	0.9	0.20	0.88	0.20	51.8
9	R2	1	0.0	0.034	9.0	LOS A	0.1	0.9	0.20	0.88	0.20	51.3
Approac	h	44	0.0	0.034	8.4	LOS A	0.1	0.9	0.20	0.88	0.20	51.7
West: Bu	uitekant Stree	et										
10	L2	1	0.0	0.052	5.5	LOS A	0.0	0.0	0.00	0.01	0.00	58.3
11	T1	98	3.0	0.052	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
Approac	h	99	3.0	0.052	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.9
All Vehic	les	366	2.4	0.114	1.5	NA	0.2	1.4	0.06	0.16	0.06	58.1

Buitekant Street and Melkboom Street

MOVEMENT SUMMARY

🕮 Site: 5 [Buitekant/Melkboom 2021 Base AM]

New Site Site Category: (None) Stop (All-Way)

Mover	nent Pe	rformance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Melkboo	m Street										
1	L2	7	3.0	0.075	11.8	LOS B	0.2	1.6	0.70	1.26	1.78	50.3
2	T1	22	0.0	0.075	11.3	LOS B	0.2	1.6	0.70	1.26	1.78	50.2
3	R2	22	3.0	0.075	11.2	LOS B	0.2	1.6	0.70	1.26	1.78	49.8
Approa	ch	52	1.7	0.075	11.4	LOS B	0.2	1.6	0.70	1.26	1.78	50.0
East: B	uitekant	Street										
4	L2	24	3.0	0.100	16.7	LOS C	0.3	2.4	0.89	1.26	2.00	47.2
5	T1	12	3.0	0.100	16.4	LOS C	0.3	2.4	0.89	1.26	2.00	47.0
6	R2	2	0.0	0.100	16.1	LOS C	0.3	2.4	0.89	1.26	2.00	46.9
Approach		38	2.8	0.100	16.6	LOS C	0.3	2.4	0.89	1.26	2.00	47.2
North: I	Melkboor	n Street										
7	L2	4	0.0	0.039	13.7	LOS B	0.1	0.8	0.81	1.25	1.85	49.3
8	T1	14	0.0	0.039	13.3	LOS B	0.1	0.8	0.81	1.25	1.85	49.0
9	R2	1	0.0	0.039	13.1	LOS B	0.1	0.8	0.81	1.25	1.85	48.8
Approa	ch	19	0.0	0.039	13.4	LOS B	0.1	0.8	0.81	1.25	1.85	49.1
West: E	Buitekant	Street										
10	L2	1	0.0	0.045	13.0	LOS B	0.1	1.0	0.78	1.26	1.83	49.7
11	T1	16	3.0	0.045	12.8	LOS B	0.1	1.0	0.78	1.26	1.83	49.3
12	R2	7	3.0	0.045	12.6	LOS B	0.1	1.0	0.78	1.26	1.83	49.1
Approa	ch	24	2.9	0.045	12.8	LOS B	0.1	1.0	0.78	1.26	1.83	49.3
All Veh	icles	133	2.0	0.100	13.4	LOS B	0.3	2.4	0.78	1.26	1.86	48.9

MOVEMENT SUMMARY

🚳 Site: 5 [Buitekant/Melkboom 2021 Base PM]

Mover	nent Per	formance - \	/ehicles									
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
U		lotal veh/h	HV %	Satn v/c	Delay sec	Service	venicles	Distance	Queued	Stop Rate	Cycles	Speed km/h
South:	Melkboor	n Street										
1	L2	7	3.0	0.076	12.9	LOS B	0.2	1.7	0.76	1.26	1.85	49.7
2	T1	18	0.0	0.076	12.4	LOS B	0.2	1.7	0.76	1.26	1.85	49.5
3	R2	19	3.0	0.076	12.3	LOS B	0.2	1.7	0.76	1.26	1.85	49.2
Approa	ch	44	1.8	0.076	12.4	LOS B	0.2	1.7	0.76	1.26	1.85	49.4
East: B	uitekant S	Street										
4	L2	38	3.0	0.135	14.2	LOS B	0.4	3.2	0.82	1.27	1.97	48.8
5	T1	24	3.0	0.135	13.9	LOS B	0.4	3.2	0.82	1.27	1.97	48.5
6	R2	7	0.0	0.135	13.5	LOS B	0.4	3.2	0.82	1.27	1.97	48.4
Approach		69	2.7	0.135	14.0	LOS B	0.4	3.2	0.82	1.27	1.97	48.7
North: I	Melkboon	n Street										
7	L2	5	0.0	0.076	14.3	LOS B	0.2	1.7	0.83	1.26	1.91	48.9
8	T1	27	0.0	0.076	13.9	LOS B	0.2	1.7	0.83	1.26	1.91	48.7
9	R2	3	0.0	0.076	13.7	LOS B	0.2	1.7	0.83	1.26	1.91	48.5
Approa	ch	36	0.0	0.076	14.0	LOS B	0.2	1.7	0.83	1.26	1.91	48.7
West: E	Buitekant	Street										
10	L2	1	0.0	0.045	12.8	LOS B	0.1	1.0	0.77	1.26	1.82	49.9
11	T1	20	3.0	0.045	12.6	LOS B	0.1	1.0	0.77	1.26	1.82	49.5
12	R2	4	3.0	0.045	12.3	LOS B	0.1	1.0	0.77	1.26	1.82	49.3
Approa	ch	25	2.9	0.045	12.5	LOS B	0.1	1.0	0.77	1.26	1.82	49.5
All Vehi	icles	175	1.9	0.135	13.4	LOS B	0.4	3.2	0.80	1.26	1.90	49.0

Site: 5 [Buitekant/Melkboom 2021 Base + Dev AM - Existing]

New Site Site Category: (None) Stop (All-Way)

Move	ment Per	formance	- Vehi	cles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	Melkboon	n Street										
1	L2	7	3.0	0.147	18.1	LOS C	0.5	3.6	0.91	1.27	2.08	46.5
2	T1	22	0.0	0.147	17.7	LOS C	0.5	3.6	0.91	1.27	2.08	46.4
3	R2	22	3.0	0.147	17.6	LOS C	0.5	3.6	0.91	1.27	2.08	46.1
Appro	ach	52	1.7	0.147	17.7	LOS C	0.5	3.6	0.91	1.27	2.08	46.3
East: I	Buitekant S	Street										
4	L2	24	3.0	0.126	12.5	LOS B	0.4	2.8	0.74	1.28	1.88	49.9
5	T1	56	3.0	0.126	12.2	LOS B	0.4	2.8	0.74	1.28	1.88	49.7
6	R2	2	0.0	0.126	11.8	LOS B	0.4	2.8	0.74	1.28	1.88	49.6
Approach		82	2.9	0.126	12.2	LOS B	0.4	2.8	0.74	1.28	1.88	49.7
North:	Melkboom	Street										
7	L2	4	0.0	0.097	26.5	LOS D	0.3	2.4	0.99	1.25	2.09	42.2
8	T1	14	0.0	0.097	26.2	LOS D	0.3	2.4	0.99	1.25	2.09	42.0
9	R2	1	0.0	0.097	25.9	LOS D	0.3	2.4	0.99	1.25	2.09	41.8
Appro	ach	19	0.0	0.097	26.2	LOS D	0.3	2.4	0.99	1.25	2.09	42.0
West:	Buitekant S	Street										
10	L2	1	0.0	0.202	11.8	LOS B	0.7	4.7	0.68	1.32	1.93	50.4
11	T1	151	3.0	0.202	11.6	LOS B	0.7	4.7	0.68	1.32	1.93	50.1
12	R2	7	3.0	0.202	11.3	LOS B	0.7	4.7	0.68	1.32	1.93	49.8
Appro	ach	159	3.0	0.202	11.5	LOS B	0.7	4.7	0.68	1.32	1.93	50.1
All Vel	hicles	312	2.6	0.202	13.6	LOS B	0.7	4.7	0.75	1.30	1.95	48.8

MOVEMENT SUMMARY

Site: 5 [Buitekant/Melkboom 2021 Base + Dev PM - Existing]

Move	ment P	erformance	- Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Melkbo	om Street										
1	L2	7	3.0	0.136	19.0	LOS C	0.5	3.4	0.92	1.27	2.08	46.1
2	T1	18	0.0	0.136	18.5	LOS C	0.5	3.4	0.92	1.27	2.08	45.9
3	R2	19	3.0	0.136	18.4	LOS C	0.5	3.4	0.92	1.27	2.08	45.7
Appro	ach	44	1.8	0.136	18.5	LOS C	0.5	3.4	0.92	1.27	2.08	45.8
East: E	Buitekan	t Street										
4	L2	38	3.0	0.260	12.6	LOS B	0.9	6.4	0.72	1.33	2.07	49.8
5	T1	149	3.0	0.260	12.3	LOS B	0.9	6.4	0.72	1.33	2.07	49.6
6	R2	7	0.0	0.260	11.9	LOS B	0.9	6.4	0.72	1.33	2.07	49.5
Approach		195	2.9	0.260	12.4	LOS B	0.9	6.4	0.72	1.33	2.07	49.6
North:	Melkboo	om Street										
7	L2	5	0.0	0.162	25.4	LOS D	0.6	4.1	0.98	1.27	2.17	42.7
8	T1	27	0.0	0.162	25.0	LOS D	0.6	4.1	0.98	1.27	2.17	42.6
9	R2	3	0.0	0.162	24.8	LOS C	0.6	4.1	0.98	1.27	2.17	42.4
Appro	ach	36	0.0	0.162	25.1	LOS D	0.6	4.1	0.98	1.27	2.17	42.6
West:	Buitekar	it Street										
10	L2	1	0.0	0.114	11.9	LOS B	0.4	2.5	0.71	1.29	1.84	50.4
11	T1	74	3.0	0.114	11.7	LOS B	0.4	2.5	0.71	1.29	1.84	50.0
12	R2	4	3.0	0.114	11.4	LOS B	0.4	2.5	0.71	1.29	1.84	49.8
Appro	ach	79	3.0	0.114	11.7	LOS B	0.4	2.5	0.71	1.29	1.84	50.0
All Vel	hicles	354	2.5	0.260	14.3	LOS B	0.9	6.4	0.77	1.31	2.03	48.4

Site: 5 [Buitekant/Melkboom 2026 AM - Existing]

New Site Site Category: (None) Stop (All-Way)

Move	Novement Performance - Vehicles Nov Turn Demand Flows Dea. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average													
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South:	Melkboom 3	Street												
1	L2	7	3.0	0.147	18.1	LOS C	0.5	3.6	0.91	1.27	2.08	46.5		
2	T1	22	0.0	0.147	17.7	LOS C	0.5	3.6	0.91	1.27	2.08	46.4		
3	R2	22	3.0	0.147	17.6	LOS C	0.5	3.6	0.91	1.27	2.08	46.1		
Approa	ach	52	1.7	0.147	17.7	LOS C	0.5	3.6	0.91	1.27	2.08	46.3		
East: E	Buitekant Str	reet												
4	L2	24	3.0	0.126	12.5	LOS B	0.4	2.8	0.74	1.28	1.88	49.9		
5	T1	56	3.0	0.126	12.2	LOS B	0.4	2.8	0.74	1.28	1.88	49.7		
6	R2	2	0.0	0.126	11.8	LOS B	0.4	2.8	0.74	1.28	1.88	49.6		
Approach		82	2.9	0.126	12.2	LOS B	0.4	2.8	0.74	1.28	1.88	49.7		
North:	Melkboom S	Street												
7	L2	4	0.0	0.097	26.5	LOS D	0.3	2.4	0.99	1.25	2.09	42.2		
8	T1	14	0.0	0.097	26.2	LOS D	0.3	2.4	0.99	1.25	2.09	42.0		
9	R2	1	0.0	0.097	25.9	LOS D	0.3	2.4	0.99	1.25	2.09	41.8		
Approa	ach	19	0.0	0.097	26.2	LOS D	0.3	2.4	0.99	1.25	2.09	42.0		
West:	Buitekant St	reet												
10	L2	1	0.0	0.202	11.8	LOS B	0.7	4.7	0.68	1.32	1.93	50.4		
11	T1	151	3.0	0.202	11.6	LOS B	0.7	4.7	0.68	1.32	1.93	50.1		
12	R2	7	3.0	0.202	11.3	LOS B	0.7	4.7	0.68	1.32	1.93	49.8		
Approa	ach	159	3.0	0.202	11.5	LOS B	0.7	4.7	0.68	1.32	1.93	50.1		
All Veh	nicles	312	2.6	0.202	13.6	LOS B	0.7	4.7	0.75	1.30	1.95	48.8		

MOVEMENT SUMMARY

Site: 5 [Buitekant/Melkboom 2026 PM - Existing]

Moven	nent Perf	ormance - V	/ehicles	i								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: I	Velkboom	Street										
1	L2	7	3.0	0.136	19.0	LOS C	0.5	3.4	0.92	1.27	2.08	46.1
2	T1	18	0.0	0.136	18.5	LOS C	0.5	3.4	0.92	1.27	2.08	45.9
3	R2	19	3.0	0.136	18.4	LOS C	0.5	3.4	0.92	1.27	2.08	45.7
Approa	ch	44	1.8	0.136	18.5	LOS C	0.5	3.4	0.92	1.27	2.08	45.8
East: B	uitekant St	reet										
4	L2	38	3.0	0.260	12.6	LOS B	0.9	6.4	0.72	1.33	2.07	49.8
5	T1	149	3.0	0.260	12.3	LOS B	0.9	6.4	0.72	1.33	2.07	49.6
6	R2	7	0.0	0.260	11.9	LOS B	0.9	6.4	0.72	1.33	2.07	49.5
Approach		195	2.9	0.260	12.4	LOS B	0.9	6.4	0.72	1.33	2.07	49.6
North: N	/lelkboom	Street										
7	L2	5	0.0	0.162	25.4	LOS D	0.6	4.1	0.98	1.27	2.17	42.7
8	T1	27	0.0	0.162	25.0	LOS D	0.6	4.1	0.98	1.27	2.17	42.6
9	R2	3	0.0	0.162	24.8	LOS C	0.6	4.1	0.98	1.27	2.17	42.4
Approa	ch	36	0.0	0.162	25.1	LOS D	0.6	4.1	0.98	1.27	2.17	42.6
West: E	uitekant S	treet										
10	L2	1	0.0	0.114	11.9	LOS B	0.4	2.5	0.71	1.29	1.84	50.4
11	T1	74	3.0	0.114	11.7	LOS B	0.4	2.5	0.71	1.29	1.84	50.0
12	R2	4	3.0	0.114	11.4	LOS B	0.4	2.5	0.71	1.29	1.84	49.8
Approa	ch	79	3.0	0.114	11.7	LOS B	0.4	2.5	0.71	1.29	1.84	50.0
All Vehi	cles	354	2.5	0.260	14.3	LOS B	0.9	6.4	0.77	1.31	2.03	48.4

Buitekant Street and Reservoir Street

MOVEMENT SUMMARY

Site: 6 [Buitekant/Reservoir 2021 Base AM]

New Site Site Category: (None) Stop (All-Way)

Movem	ent Perfor	mance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	itekant Stree	et										
5	T1	1	3.0	0.022	9.7	LOS A	0.1	0.4	0.53	1.23	1.55	51.1
6	R2	20	0.0	0.022	9.3	LOS A	0.1	0.4	0.53	1.23	1.55	51.0
Approad	Approach North: Reservoir Stree		0.2	0.022	9.3	LOS A	0.1	0.4	0.53	1.23	1.55	51.0
North: R	eservoir Stre	eet										
7	L2	23	0.0	0.052	14.2	LOS B	0.2	1.1	0.83	1.24	1.88	48.7
9	R2	1	0.0	0.052	13.6	LOS B	0.2	1.1	0.83	1.24	1.88	48.3
Approad	:h	24	0.0	0.052	14.1	LOS B	0.2	1.1	0.83	1.24	1.88	48.7
West: B	uitekant Stre	et										
10	L2	1	0.0	0.005	14.2	LOS B	0.0	0.1	0.83	1.24	1.84	48.8
11	T1	1	3.0	0.005	14.0	LOS B	0.0	0.1	0.83	1.24	1.84	48.5
Approad	11 T1 Approach		1.5	0.005	14.1	LOS B	0.0	0.1	0.83	1.24	1.84	48.7
All Vehic	cles	47	0.1	0.052	12.0	LOS B	0.2	1.1	0.69	1.24	1.73	49.7

MOVEMENT SUMMARY

Site: 6 [Buitekant/Reservoir 2021 Base PM]

Moverr	nent Perforr	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	uitekant Stree	et										
5	T1	1	3.0	0.034	9.3	LOS A	0.1	0.6	0.46	1.24	1.50	51.2
6	R2	35	0.0	0.034	9.0	LOS A	0.1	0.6	0.46	1.24	1.50	51.1
Approach North: Desenvoir Stre		36	0.1	0.034	9.0	LOS A	0.1	0.6	0.46	1.24	1.50	51.1
North: Reservoir Stre		et										
7	L2	25	0.0	0.077	17.4	LOS C	0.3	1.8	0.90	1.25	1.98	46.8
9	R2	1	0.0	0.077	16.8	LOS C	0.3	1.8	0.90	1.25	1.98	46.4
Approad	ch	26	0.0	0.077	17.4	LOS C	0.3	1.8	0.90	1.25	1.98	46.8
West: B	uitekant Stre	et										
10	L2	1	0.0	0.004	13.2	LOS B	0.0	0.1	0.79	1.24	1.80	49.5
11	T1	1	3.0	0.004	13.0	LOS B	0.0	0.1	0.79	1.24	1.80	49.1
Approa	ch	2	1.5	0.004	13.1	LOS B	0.0	0.1	0.79	1.24	1.80	49.3
All Vehi	cles	64	0.1	0.077	12.6	LOS B	0.3	1.8	0.66	1.24	1.71	49.2

Site: 6 [Buitekant/Reservoir 2021 Base + Dev AM - Existing]

New Site Site Category: (None) Stop (All-Way)

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: E	Buitekant S	Street										
5	T1	44	3.0	0.060	9.4	LOS A	0.2	1.1	0.46	1.28	1.52	51.2
6	R2	20	0.0	0.060	9.0	LOS A	0.2	1.1	0.46	1.28	1.52	51.1
Approa	ach	64	2.1	0.060	9.3	LOS A	0.2	1.1	0.46	1.28	1.52	51.2
North: Reservoir		Street										
7	L2	23	0.0	0.087	20.2	LOS C	0.3	2.1	0.94	1.25	2.03	45.2
9	R2	1	0.0	0.087	19.6	LOS C	0.3	2.1	0.94	1.25	2.03	44.9
Approa	ach	24	0.0	0.087	20.1	LOS C	0.3	2.1	0.94	1.25	2.03	45.2
West:	Buitekant	Street										
10	L2	1	0.0	0.127	9.9	LOS A	0.3	2.5	0.48	1.32	1.62	51.5
11	T1	135	3.0	0.127	9.7	LOS A	0.3	2.5	0.48	1.32	1.62	51.1
Approa	ach	136	3.0	0.127	9.7	LOS A	0.3	2.5	0.48	1.32	1.62	51.1
All Veh	nicles	224	2.4	0.127	10.7	LOS B	0.3	2.5	0.52	1.30	1.64	50.4

MOVEMENT SUMMARY

Site: 6 [Buitekant/Reservoir 2021 Base + Dev PM - Existing]

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: E	Buitekant S	street										
5	T1	125	3.0	0.135	9.3	LOS A	0.4	2.5	0.41	1.31	1.56	51.2
6	R2	35	0.0	0.135	8.9	LOS A	0.4	2.5	0.41	1.31	1.56	51.1
Approa	ach	160	2.3	0.135	9.2	LOS A	0.4	2.5	0.41	1.31	1.56	51.2
North:	Reservoir	Street										
7	L2	25	0.0	0.101	21.3	LOS C	0.3	2.4	0.95	1.25	2.06	44.6
9	R2	1	0.0	0.101	20.7	LOS C	0.3	2.4	0.95	1.25	2.06	44.2
Approa	ach	26	0.0	0.101	21.3	LOS C	0.3	2.4	0.95	1.25	2.06	44.6
West: I	Buitekant S	Street										
10	L2	1	0.0	0.055	9.9	LOS A	0.1	1.0	0.51	1.29	1.57	51.5
11	T1	54	3.0	0.055	9.7	LOS A	0.1	1.0	0.51	1.29	1.57	51.1
Approa	ach	55	2.9	0.055	9.7	LOS A	0.1	1.0	0.51	1.29	1.57	51.1
All Veh	icles	241	2.2	0.135	10.6	LOS B	0.4	2.5	0.49	1.30	1.62	50.4

Site: 6 [Buitekant/Reservoir 2026 AM - Existing]

New Site Site Category: (None) Stop (All-Way)

Moven	ient Perfori	mance - V	ehicles									
Mov ID	Tum	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: B	uitekant Stree	et										
5	T1	44	3.0	0.060	9.4	LOS A	0.2	1.1	0.46	1.28	1.52	51.2
6	R2	20	0.0	0.060	9.0	LOS A	0.2	1.1	0.46	1.28	1.52	51.1
Approa	ch	64	2.1	0.060	9.3	LOS A	0.2	1.1	0.46	1.28	1.52	51.2
North: Reservoir Str		eet										
7	L2	23	0.0	0.087	20.2	LOS C	0.3	2.1	0.94	1.25	2.03	45.2
9	R2	1	0.0	0.087	19.6	LOS C	0.3	2.1	0.94	1.25	2.03	44.9
Approa	ch	24	0.0	0.087	20.1	LOS C	0.3	2.1	0.94	1.25	2.03	45.2
West: B	uitekant Stre	et										
10	L2	1	0.0	0.127	9.9	LOS A	0.3	2.5	0.48	1.32	1.62	51.5
11	T1	135	3.0	0.127	9.7	LOS A	0.3	2.5	0.48	1.32	1.62	51.1
Approa	ch	136	3.0	0.127	9.7	LOS A	0.3	2.5	0.48	1.32	1.62	51.1
All Vehi	cles	224	2.4	0.127	10.7	LOS B	0.3	2.5	0.52	1.30	1.64	50.4

MOVEMENT SUMMARY

Site: 6 [Buitekant/Reservoir 2026 PM - Existing]

Movem	ent Perforr	nance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Bu	iitekant Stree	ŧ										
5	T1	125	3.0	0.135	9.3	LOS A	0.4	2.5	0.41	1.31	1.56	51.2
6	R2	35	0.0	0.135	8.9	LOS A	0.4	2.5	0.41	1.31	1.56	51.1
Approad	h	160	2.3	0.135	9.2	LOS A	0.4	2.5	0.41	1.31	1.56	51.2
North: R	North: Reservoir Stre											
7	L2	25	0.0	0.101	21.3	LOS C	0.3	2.4	0.95	1.25	2.06	44.6
9	R2	1	0.0	0.101	20.7	LOS C	0.3	2.4	0.95	1.25	2.06	44.2
Approac	h	26	0.0	0.101	21.3	LOS C	0.3	2.4	0.95	1.25	2.06	44.6
West: B	uitekant Stree	et										
10	L2	1	0.0	0.055	9.9	LOS A	0.1	1.0	0.51	1.29	1.57	51.5
11	T1	54	3.0	0.055	9.7	LOS A	0.1	1.0	0.51	1.29	1.57	51.1
Approac	h	55	2.9	0.055	9.7	LOS A	0.1	1.0	0.51	1.29	1.57	51.1
All Vehic	les	241	2.2	0.135	10.6	LOS B	0.4	2.5	0.49	1.30	1.62	50.4

Buitekant Street and Road 1

MOVEMENT SUMMARY

Site: 7 [Buitekant/Road 1 2021 Base + Dev AM - Proposed]

New Site Site Category: (None) Stop (Two-Way)

Move	ment Per	formance	- Vehic	les								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 1											
1	L2	1	0.0	0.066	5.5	LOS A	0.0	0.0	0.00	0.00	0.00	58.3
2	T1	135	0.0	0.066	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Approa	ach	136	0.0	0.066	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
North:	Buitekant	Street										
8	T1	44	0.0	0.023	0.0	LOS A	0.0	0.0	0.01	0.01	0.01	59.8
9	R2	1	0.0	0.023	5.8	LOS A	0.0	0.0	0.01	0.01	0.01	57.6
Approa	ach	45	0.0	0.023	0.1	NA	0.0	0.0	0.01	0.01	0.01	59.8
West: I	Buitekant	Street										
10	L2	1	0.0	0.002	8.5	LOS A	0.0	0.0	0.24	0.85	0.24	51.8
12	R2	1	3.0	0.002	8.3	LOS A	0.0	0.0	0.24	0.85	0.24	51.2
Approa	ach	2	1.5	0.002	8.4	LOS A	0.0	0.0	0.24	0.85	0.24	51.5
All Veh	nicles	183	0.0	0.066	0.2	NA	0.0	0.0	0.01	0.02	0.01	59.8

MOVEMENT SUMMARY

Site: 7 [Buitekant/Road 1 2021 Base + Dev PM - Proposed]

Mover	nent Per	formance	- Vehi	cles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 1											
1	L2	1	3.0	0.027	5.6	LOS A	0.0	0.0	0.00	0.01	0.00	58.1
2	T1	54	0.0	0.027	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
Approa	ich	55	0.1	0.027	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.9
North:	Buitekant	Street										
8	T1	125	0.0	0.065	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
9	R2	1	0.0	0.065	5.6	LOS A	0.0	0.0	0.00	0.01	0.00	57.7
Approa	ich	126	0.0	0.065	0.0	NA	0.0	0.0	0.00	0.01	0.00	59.9
West: E	Buitekant	Street										
10	L2	1	0.0	0.002	8.2	LOS A	0.0	0.0	0.15	0.90	0.15	51.8
12	R2	1	3.0	0.002	8.3	LOS A	0.0	0.0	0.15	0.90	0.15	51.2
Approa	ich	2	1.5	0.002	8.2	LOS A	0.0	0.0	0.15	0.90	0.15	51.5
All Veh	icles	183	0.0	0.065	0.2	NA	0.0	0.0	0.00	0.02	0.00	59.8

Site: 7 [Buitekant/Road 1 2026 AM - Proposed]

New Site Site Category: (None) Stop (Two-Way)

Moven	nent Perfo	rmance - V	/ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 1											
1	L2	1	0.0	0.066	5.5	LOS A	0.0	0.0	0.00	0.00	0.00	58.3
2	T1	135	0.0	0.066	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Approach North: Buitekant Stra		136	0.0	0.066	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
North: Buitekant Stre		reet										
8	T1	44	0.0	0.023	0.0	LOS A	0.0	0.0	0.01	0.01	0.01	59.8
9	R2	1	0.0	0.023	5.8	LOS A	0.0	0.0	0.01	0.01	0.01	57.6
Approa	ch	45	0.0	0.023	0.1	NA	0.0	0.0	0.01	0.01	0.01	59.8
West: E	Buitekant Str	eet										
10	L2	1	0.0	0.002	8.5	LOS A	0.0	0.0	0.24	0.85	0.24	51.8
12	R2	1	3.0	0.002	8.3	LOS A	0.0	0.0	0.24	0.85	0.24	51.2
Approa	ch	2	1.5	0.002	8.4	LOS A	0.0	0.0	0.24	0.85	0.24	51.5
All Vehi	cles	183	0.0	0.066	0.2	NA	0.0	0.0	0.01	0.02	0.01	59.8

MOVEMENT SUMMARY

Site: 7 [Buitekant/Road 1 2026 PM - Proposed]

Movem	ent Perfor	mance - V	/ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: F	Road 1											
1	L2	1	3.0	0.027	5.6	LOS A	0.0	0.0	0.00	0.01	0.00	58.1
2	T1	54	0.0	0.027	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
Approac	:h	55	0.1	0.027	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.9
North: B	uitekant Stre	eet										
8	T1	125	0.0	0.065	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
9	R2	1	0.0	0.065	5.6	LOS A	0.0	0.0	0.00	0.01	0.00	57.7
Approac	:h	126	0.0	0.065	0.0	NA	0.0	0.0	0.00	0.01	0.00	59.9
West: Bi	uitekant Stre	et										
10	L2	1	0.0	0.002	8.2	LOS A	0.0	0.0	0.15	0.90	0.15	51.8
12	R2	1	3.0	0.002	8.3	LOS A	0.0	0.0	0.15	0.90	0.15	51.2
Approac	:h	2	1.5	0.002	8.2	LOS A	0.0	0.0	0.15	0.90	0.15	51.5
All Vehic	les	183	0.0	0.065	0.2	NA	0.0	0.0	0.00	0.02	0.00	59.8

Road 1 and Road 2

MOVEMENT SUMMARY

Site: 8 [Road 1/Road 2 2021 Base + Dev AM - Proposed]

New Site Site Category: (None) Stop (Two-Way)

Move	ment Per	formance	- Vehio	cles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 1											
1	L2	1	3.0	0.001	5.6	LOS A	0.0	0.0	0.00	0.30	0.00	55.8
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.00	0.30	0.00	57.4
Approa	ach	2	1.5	0.001	2.8	NA	0.0	0.0	0.00	0.30	0.00	56.6
North: Road 1												
8	T1	1	0.0	0.025	0.0	LOS A	0.1	0.8	0.02	0.58	0.02	55.1
9	R2	44	0.0	0.025	5.5	LOS A	0.1	0.8	0.02	0.58	0.02	53.2
Approa	ach	45	0.0	0.025	5.3	NA	0.1	0.8	0.02	0.58	0.02	53.2
West:	Road 2											
10	L2	135	0.0	0.095	8.0	LOS A	0.4	2.8	0.01	0.99	0.01	51.8
12	R2	1	3.0	0.095	7.8	LOS A	0.4	2.8	0.01	0.99	0.01	51.1
Approa	ach	136	0.0	0.095	8.0	LOS A	0.4	2.8	0.01	0.99	0.01	51.7
All Vel	nicles	183	0.0	0.095	7.3	NA	0.4	2.8	0.01	0.88	0.01	52.2

MOVEMENT SUMMARY

Site: 8 [Road 1/Road 2 2021 Base + Dev PM - Proposed]

Mover	nent Per	formance	- Vehic	:les								
Mov ID	Tum	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 1											
1	L2	1	3.0	0.001	5.6	LOS A	0.0	0.0	0.00	0.30	0.00	55.8
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.00	0.30	0.00	57.4
Approa	ich	2	1.5	0.001	2.8	NA	0.0	0.0	0.00	0.30	0.00	56.6
North:	Road 1											
8	T1	1	0.0	0.070	0.0	LOS A	0.3	2.3	0.02	0.59	0.02	55.0
9	R2	125	0.0	0.070	5.5	LOS A	0.3	2.3	0.02	0.59	0.02	53.1
Approa	ich	126	0.0	0.070	5.4	NA	0.3	2.3	0.02	0.59	0.02	53.1
West: F	Road 2											
10	L2	54	0.0	0.038	8.0	LOS A	0.2	1.1	0.01	0.99	0.01	51.8
12	R2	1	3.0	0.038	8.1	LOS A	0.2	1.1	0.01	0.99	0.01	51.1
Approa	ich	55	0.1	0.038	8.0	LOS A	0.2	1.1	0.01	0.99	0.01	51.7
All Veh	icles	183	0.0	0.070	6.2	NA	0.3	2.3	0.02	0.70	0.02	52.7

Site: 8 [Road 1/Road 2 2026 AM - Proposed]

New Site Site Category: (None) Stop (Two-Way)

Moven	nent Perfo	rmance - V	/ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 1											
1	L2	1	3.0	0.001	5.6	LOS A	0.0	0.0	0.00	0.30	0.00	55.8
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.00	0.30	0.00	57.4
Approa	ch	2	1.5	0.001	2.8	NA	0.0	0.0	0.00	0.30	0.00	56.6
North: F	Road 1											
8	T1	1	0.0	0.025	0.0	LOS A	0.1	0.8	0.02	0.58	0.02	55.1
9	R2	44	0.0	0.025	5.5	LOS A	0.1	0.8	0.02	0.58	0.02	53.2
Approa	ch	45	0.0	0.025	5.3	NA	0.1	0.8	0.02	0.58	0.02	53.2
West: F	load 2											
10	L2	135	0.0	0.095	8.0	LOS A	0.4	2.8	0.01	0.99	0.01	51.8
12	R2	1	3.0	0.095	7.8	LOS A	0.4	2.8	0.01	0.99	0.01	51.1
Approa	ch	136	0.0	0.095	8.0	LOS A	0.4	2.8	0.01	0.99	0.01	51.7
All Vehi	cles	183	0.0	0.095	7.3	NA	0.4	2.8	0.01	0.88	0.01	52.2

MOVEMENT SUMMARY

Site: 8 [Road 1/Road 2 2026 PM - Proposed]

Movern	ent Perfor	mance - V	/ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: F	Road 1											
1	L2	1	3.0	0.001	5.6	LOS A	0.0	0.0	0.00	0.30	0.00	55.8
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.00	0.30	0.00	57.4
Approad	:h	2	1.5	0.001	2.8	NA	0.0	0.0	0.00	0.30	0.00	56.6
North: Road 1												
8	T1	1	0.0	0.070	0.0	LOS A	0.3	2.3	0.02	0.59	0.02	55.0
9	R2	125	0.0	0.070	5.5	LOS A	0.3	2.3	0.02	0.59	0.02	53.1
Approad	:h	126	0.0	0.070	5.4	NA	0.3	2.3	0.02	0.59	0.02	53.1
West: R	oad 2											
10	L2	54	0.0	0.038	8.0	LOS A	0.2	1.1	0.01	0.99	0.01	51.8
12	R2	1	3.0	0.038	8.1	LOS A	0.2	1.1	0.01	0.99	0.01	51.1
Approad	:h	55	0.1	0.038	8.0	LOS A	0.2	1.1	0.01	0.99	0.01	51.7
All Vehic	cles	183	0.0	0.070	6.2	NA	0.3	2.3	0.02	0.70	0.02	52.7

Road 2 and Road 3

MOVEMENT SUMMARY

Site: 9 [Road 2/Road 3 2021 Base + Dev AM - Proposed]

New Site Site Category: (None) Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 3											
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.05	0.29	0.05	57.2
3	R2	1	0.0	0.001	5.5	LOS A	0.0	0.0	0.05	0.29	0.05	55.2
Approa	ach	2	0.0	0.001	2.8	NA	0.0	0.0	0.05	0.29	0.05	56.2
East: Road 2												
4	L2	1	0.0	0.004	8.0	LOS A	0.0	0.1	0.02	0.98	0.02	52.1
6	R2	4	0.0	0.004	7.5	LOS A	0.0	0.1	0.02	0.98	0.02	51.6
Approa	ach	5	0.0	0.004	7.6	LOS A	0.0	0.1	0.02	0.98	0.02	51.7
North:	Road 3											
7	L2	14	0.0	0.008	5.5	LOS A	0.0	0.0	0.00	0.54	0.00	53.9
8	T1	1	0.0	0.008	0.0	LOS A	0.0	0.0	0.00	0.54	0.00	55.3
Approa	ach	15	0.0	0.008	5.1	NA	0.0	0.0	0.00	0.54	0.00	54.0
All Veh	nicles	22	0.0	0.008	5.5	NA	0.0	0.1	0.01	0.62	0.01	53.6

MOVEMENT SUMMARY

Site: 9 [Road 2/Road 3 2021 Base + Dev PM - Proposed]

Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 3											
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.03	0.30	0.03	57.3
3	R2	1	0.0	0.001	5.5	LOS A	0.0	0.0	0.03	0.30	0.03	55.2
Approa	ich	2	0.0	0.001	2.7	NA	0.0	0.0	0.03	0.30	0.03	56.2
East: Road 2												
4	L2	1	0.0	0.012	8.0	LOS A	0.0	0.3	0.03	0.98	0.03	52.1
6	R2	13	0.0	0.012	7.5	LOS A	0.0	0.3	0.03	0.98	0.03	51.6
Approa	ich	14	0.0	0.012	7.5	LOS A	0.0	0.3	0.03	0.98	0.03	51.6
North:	Road 3											
7	L2	5	0.0	0.003	5.5	LOS A	0.0	0.0	0.00	0.49	0.00	54.4
8	T1	1	0.0	0.003	0.0	LOS A	0.0	0.0	0.00	0.49	0.00	55.8
Approa	ich	6	0.0	0.003	4.6	NA	0.0	0.0	0.00	0.49	0.00	54.6
All Veh	icles	22	0.0	0.012	6.2	NA	0.0	0.3	0.02	0.77	0.02	52.9

Site: 9 [Road 2/Road 3 2026 AM - Proposed]

New Site Site Category: (None) Stop (Two-Way)

Moven	nent Perfo	rmance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Road 3											
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.05	0.29	0.05	57.2
3	R2	1	0.0	0.001	5.5	LOS A	0.0	0.0	0.05	0.29	0.05	55.2
Approa	ch	2	0.0	0.001	2.8	NA	0.0	0.0	0.05	0.29	0.05	56.2
East: R	load 2											
4	L2	1	0.0	0.004	8.0	LOS A	0.0	0.1	0.02	0.98	0.02	52.1
6	R2	4	0.0	0.004	7.5	LOS A	0.0	0.1	0.02	0.98	0.02	51.6
Approa	ch	5	0.0	0.004	7.6	LOS A	0.0	0.1	0.02	0.98	0.02	51.7
North: I	Road 3											
7	L2	14	0.0	0.008	5.5	LOS A	0.0	0.0	0.00	0.54	0.00	53.9
8	T1	1	0.0	0.008	0.0	LOS A	0.0	0.0	0.00	0.54	0.00	55.3
Approa	ch	15	0.0	0.008	5.1	NA	0.0	0.0	0.00	0.54	0.00	54.0
All Vehi	icles	22	0.0	0.008	5.5	NA	0.0	0.1	0.01	0.62	0.01	53.6

MOVEMENT SUMMARY

Site: 9 [Road 2/Road 3 2026 PM - Proposed]

Movem	ent Perfor	mance - V	ehicles									
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: Road 3												
2	T1	1	0.0	0.001	0.0	LOS A	0.0	0.0	0.03	0.30	0.03	57.3
3	R2	1	0.0	0.001	5.5	LOS A	0.0	0.0	0.03	0.30	0.03	55.2
Approac	h	2	0.0	0.001	2.7	NA	0.0	0.0	0.03	0.30	0.03	56.2
East: Road 2												
4	L2	1	0.0	0.012	8.0	LOS A	0.0	0.3	0.03	0.98	0.03	52.1
6	R2	13	0.0	0.012	7.5	LOS A	0.0	0.3	0.03	0.98	0.03	51.6
Approac	h	14	0.0	0.012	7.5	LOS A	0.0	0.3	0.03	0.98	0.03	51.6
North: R	oad 3											
7	L2	5	0.0	0.003	5.5	LOS A	0.0	0.0	0.00	0.49	0.00	54.4
8	T1	1	0.0	0.003	0.0	LOS A	0.0	0.0	0.00	0.49	0.00	55.8
Approac	h	6	0.0	0.003	4.6	NA	0.0	0.0	0.00	0.49	0.00	54.6
All Vehic	les	22	0.0	0.012	6.2	NA	0.0	0.3	0.02	0.77	0.02	52.9

Redefining exceptional

Through our specialist expertise, we are challenging boundaries to deliver advanced infrastructure solutions.